

1 **Relating Hydraulic-Electrical-Elastic Properties of Natural Rock Fractures at**
2 **Elevated Stress and Associated Transient Changes of Fracture Flow**

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19 **Highlights:**

- 20 • Changes in permeability and resistivity with stress depend on pore connectivity and are
21 less sensitive to the fracture roughness.
- 22 • Changes in velocity with stress depend on the spatial distribution of asperity contacts and
23 roughness dependency of porosity.
- 24 • Resistivity can be linked with permeability and flow area regardless of fracture
25 roughness, whereas velocity has roughness dependence.
- 26 • Transitions of fracture flow patterns with stress define three stages that may be evident in
27 remotely measured geophysical properties.
28

29 **Abstract**

30 Monitoring the hydraulic properties within subsurface fractures is vitally important in the contexts of
31 geoen지니어ing developments and seismicity. Geophysical observations are promising tools for remote
32 determination of subsurface hydraulic properties; however, quantitative interpretations are hampered by the
33 paucity of relevant geophysical data for fractured rock masses. This study explores simultaneous changes
34 in hydraulic and geophysical properties of natural rock fractures with increasing normal stress and
35 correlates these property changes through coupling experiments and digital fracture simulations. Our lattice
36 Boltzmann simulation reveals transitions in three-dimensional flow paths, and finite-element modeling
37 enables us to investigate the corresponding evolution of geophysical properties. We show that electrical
38 resistivity is linked with permeability and flow area regardless of fracture roughness, whereas elastic wave
39 velocity is roughness dependent. This discrepancy arises from the different sensitivities of these quantities
40 to microstructure: velocity is sensitive to the spatial distribution of asperity contacts, whereas permeability
41 and resistivity are insensitive to contact distribution but instead are controlled by fluid connectivity. We
42 also are able to categorize fracture flow patterns as aperture-dependent, aperture-independent, or
43 disconnected flows, with transitions at specific stress levels. Elastic wave velocity offers potential for
44 detecting the transition between aperture-dependent flow and aperture-independent flow, and resistivity is
45 sensitive to the state of connection of the fracture flow. The hydraulic-electrical-elastic relationships
46 reported here may be beneficial for improving geophysical interpretations and may find applications in
47 studies of seismogenic zones and geothermal reservoirs.

48 Keywords: fracture flow, permeability, elastic wave velocity, resistivity, lattice Boltzmann method, digital
49 rock physics

50 **1 Introduction**

51 The hydraulic properties of fractured geological formations have been of interest for many purposes
52 such as developing fluid resources (e.g., geothermal fluids, shale oil and groundwater), geological storage
53 or disposal, and seismic events (fault reactivation and induced seismicity). It is known that fracture
54 permeability and preferential flow paths within fractures are controlled by the heterogeneous distribution
55 of apertures, which can vary as stress changes (Krantz et al. 1979; Raven and Gale 1985; Thompson and
56 Brown 1991; Watanabe et al. 2008; Ishibashi et al. 2015; Chen et al. 2017; Vogler et al. 2018). In-situ stress
57 is never constant during geoen지니어ing developments or on the geological time scale, and consequently
58 the aperture distribution and associated hydraulic properties also must change in natural settings (e.g.,
59 Manga et al. 2012). These changes produce transitions in the patterns of fracture flow that in turn control
60 the fault reactivation cycle (Sibson et al. 1988) and characterize the transport behavior of fluid resources.

61 Geophysical observations can detect changes in electrical resistivity or elastic wave velocity that
62 may reflect subsurface stress changes associated with hydraulic stimulation, earthquakes or geothermal
63 fluid production (Peacock et al. 2012, 2013; Didana et al. 2017; Mazzella and Morrison 1974; Park 1991;
64 Gunasekera et al. 2003; Brenguier et al. 2008; Nimiya et al. 2017; Taira et al. 2018). It would be beneficial
65 if changes in aperture-related hydraulic properties triggered by subsurface stress changes could be linked
66 to geophysical properties that can be remotely monitored. Studies based on synthetic or simulated single
67 fractures have related hydraulic properties to electrical properties (Stesky 1986; Brown 1989; Volik et al.
68 1997; Kirkby et al. 2016) and to elastic properties (Pyrak-Nolte and Morris 2000; Petrovitch et al. 2013,
69 2014; Pyrak-Nolte and Nolte 2016; Wang and Cardenas 2016). These studies have confirmed that the
70 relationships of these properties depend on features of the fracture microstructure (e.g., pore connectivity,
71 tortuosity, apertures and contacts), which varies with the initial fracture roughness and changes with normal
72 stress. On the one hand, connected apertures are characterized by pore connectivity and tortuosity, both of
73 which are strongly related to the permeability-resistivity relationship. On the other hand, discrete points of
74 contact (asperities) contribute to hydro-mechanical properties. Therefore, both hydraulic-electrical and
75 hydraulic-elastic relations may reflect similar microstructures; however, the underlying mechanisms do not
76 necessarily have a mutual correlation. Simultaneous measurements in identical samples may shed light on
77 the nature of variations in rock properties and their relationships. To our knowledge, no study has
78 simultaneously investigated hydraulic, electrical and elastic properties of natural rock fractures.

79 This study took advantage of recent advances in Digital Rock Physics (e.g., Tsuji et al. 2019; Sain
80 et al. 2014) that enabled us to simultaneously determine multiple properties in the same sample while
81 visualizing its microstructure. In this study, we explored the simultaneous changes in fracture permeability,
82 electrical resistivity and elastic wave velocity of natural rock fractures that occur with increasing normal
83 stress. By coupling experiments and digital fracture simulations, we investigated the correlations between
84 hydraulic-electrical-elastic properties and addressed their governing mechanisms. Many studies have
85 reported a correlation between permeability and fracture specific stiffness, which is related to the amplitude
86 of the seismic response (i.e., attenuation), but have not established a direct correlation between permeability
87 and seismic velocity (Pyrak-Nolte and Nolte 2016; Wang and Cardenas 2016). Some experimental studies
88 have observed velocity changes with aperture closure (e.g., Nara et al. 2011; Choi et al. 2013), but none
89 has established a direct relationship between seismic velocity and fracture permeability. As an alternative
90 to fracture specific stiffness, in this study we adopted finite-element modeling of static elasticity to calculate
91 elastic wave velocity. In this paper, we also evaluate the local behavior of the fluid flow (i.e., preferential
92 flow paths) within fractures to investigate the connectivity of flow paths, flow area and their transient
93 changes. Our lattice Boltzmann simulation of digitized rock fractures reveals transitions in 3D fracture flow
94 patterns that accompany stress changes, which are difficult to observe in laboratory experiments or in the

95 field. We discuss how transient changes of the fracture flow pattern are correlated with hydraulic and
96 geophysical properties and suggest possible applications of our findings to seismogenic zones and
97 geothermal reservoirs.

98 **2 Methods**

99 **2.1 Sample and Experimental Procedure**

100 We evaluated the dependency of the fracture permeability on the effective normal stress in fluid
101 flow experiments. These employed two cylindrical fractured samples of Inada granite 50 mm in diameter
102 and 80 mm long, in which the fracture plane was parallel to the central axis. The two samples differed in
103 the roughness characteristics of their fracture surfaces, as determined from the surface topographies of the
104 hanging wall and footwall which we mapped in a grid of cells 23.433 μm square with a 3D measuring
105 microscope (Keyence, VR-3050). The surface of one sample, called the smooth fracture hereafter, had a
106 fractal dimension of 2.5 and a **root mean square (rms) roughness** of 1.3 mm, whereas the surface of the
107 other sample, called the rough fracture hereafter, had a fractal dimension of 2.4 and **rms roughness** of 1.7
108 mm (Power et al. 1987; Power and Durham 1997). **The fractal dimension describes the scaling**
109 **characteristics of surface topographies and is a measure of fracture surface roughness (Brown, 1995). The**
110 **rms roughness, called roughness hereafter, represents an rms height of fracture surface topographies.** The
111 initial aperture distribution of each fracture and the corresponding probability histogram are shown in Fig.
112 1. Note that initial aperture models are created by numerically **matting the mapped fracture surfaces**, where
113 they are assumed to be in contact at one point. The aperture distribution in the smooth fracture (Fig. 1a)
114 shows less spatial variation than that of the rough fracture (Fig. 1b). Consequently, the aperture distribution
115 of the smooth fracture shows a sharp peak of probability whereas the rough fracture shows a broad
116 distribution (Fig. 1c).

117 **After measuring the fracture surfaces, we conducted fluid flow experiments on these two samples.**
118 **Distilled water was injected into jacketed samples under various effective normal stresses between 5 and**
119 **30 MPa. The pore-inlet, pore-outlet, and confining-oil pressures were independently controlled by syringe**
120 **pumps. For each stress state, we measured flow rates and thereby evaluated the fracture permeability based**
121 **on the cubic law (e.g., Witherspoon et al. 1980), where we assumed Darcy flow and a negligible matrix**
122 **permeability of granite (between 10^{-19} and 10^{-22} m^2).**

123 2.2 Numerical Simulation

124 We performed a series of numerical simulations on digitized fractures. Three-dimensional digital
125 fracture models were prepared for each sample directly from the mapped surface topographies described
126 above in a system of 0.1 mm cubic voxels. The use of a three-dimensional fractured sample enabled us to
127 model local transport properties along with the rough-walled fractures. Although the voxel size potentially
128 affects absolute values of permeability and resistivity to some degree, we confirmed that a 0.1 mm voxel
129 system is small enough for our qualitative interpretations (Appendix 1). The distance between the two
130 surfaces was adjusted in each model by uniformly reducing the local apertures so that the digitized fracture
131 had a simulated permeability equivalent to that measured in the real fractures (Watanabe et al. 2008;
132 Ishibashi et al. 2015).

133 Subsequently, we simulated 3D local flows within the fractures by the lattice Boltzmann method,
134 which is suitable for modeling heterogeneous local flows with complex boundaries (He and Luo 1997;
135 Jiang et al. 2014). The governing equation for the lattice Boltzmann method in the D3Q19 model is given
136 by (Ahrenholz et al. 2008)

$$137 \quad f_i(\mathbf{x} + \mathbf{e}_i \Delta t, t + \Delta t) = f_i(\mathbf{x}, t) + \mathbf{\Omega}_i, \quad i = 0, \dots, 18, \quad (1)$$

138 where Δt is the time step and $f_i(\mathbf{x}, t)$ is the particle distribution function that represents the probability of
139 finding a particle at node \mathbf{x} and time t with velocity \mathbf{e}_i . Collision operators $\mathbf{\Omega}$ are defined by

$$140 \quad \mathbf{\Omega} = \mathbf{M}^{-1} \mathbf{S}[(\mathbf{M} \mathbf{f}) - \mathbf{m}^{eq}], \quad (2)$$

141 where \mathbf{M} is a transformation matrix that transforms the particle distributions into moment space. The
142 equilibrium vector \mathbf{m}^{eq} is composed of equilibrium moments, and the matrix \mathbf{S} is a diagonal collision
143 matrix indicating the relaxation rates (Jiang et al. 2014). We implemented this model using advanced
144 memory-saving schemes and parallel-GPU techniques to simulate digital fracture systems with a large
145 domain and high resolution (Jiang et al. 2014). At the fracture surfaces, bounce-back boundaries (a no-slip
146 scheme at fluid-solid interfaces) were implemented. Provision of a constant body force from the inlet to the
147 outlet boundaries and the periodic boundary along the fracture plane enabled us to simulate the fracture
148 flow (Fig. 2). Permeability along the fracture was estimated from the macroscopic flow velocity that was
149 calculated from the particle distribution function (f_i). A series of lattice Boltzmann simulations enabled us
150 to explore the changes with stress state in permeability and in the flow area, defined as the ratio of the area
151 of preferential flow paths to the area of the fracture plane (Watanabe et al. 2009).

152 Once the lattice Boltzmann simulations yielded estimates of the heterogeneous distribution of flow
 153 within the fracture, we evaluated both the resistivity and the elastic wave velocity by using the finite-
 154 element method, which is a well-established method of computing rock properties from three-dimensional
 155 microstructure (Garboczi 1998; Andrä et al. 2013; Saxena and Mavko 2016). **Both analyses implemented**
 156 **a periodic boundary along the fracture plane.** Resistivity in the direction parallel to the fracture plane (and
 157 the fluid-flow direction) was calculated from Ohm's law, where the electric current was simulated from the
 158 potential difference between the inlet and outlet boundaries. Parameters used in our finite-element modeling
 159 are summarized in Table 1. For the electrical conductivity of the solid, we used the experimental value of
 160 Inada granite under dry conditions, measured by the four-electrode method with an impedance analyzer
 161 (Solartron Analytical, SI 1260A) at 10 mHz.

162 Elastic wave velocity in the direction perpendicular to the fracture plane was estimated from the
 163 simulated static elasticity under the triaxial stress state. The finite-element analysis of static elasticity
 164 enabled us to simulate the elastic wave velocity under the low-frequency limit, where a wavelength much
 165 longer than the fracture aperture was assumed. The linear stress-strain relationship is expressed as Hooke's
 166 law:

$$167 \quad \sigma_i = C_{ij} \varepsilon_j, \quad i, j = 1, \dots, 6, \quad (3)$$

168 where C_{ij} is the **stiffness tensor** (in Voigt notation). σ_i and ε_j are stress and strain tensors, both of which
 169 are solved in a finite-element analysis associated with engineered **strain** (Garboczi 1998). Because our
 170 fracture models can be assumed to be transversely isotropic material along the z -axis (perpendicular to the
 171 fracture plane), C_{ij} has five independent elements (Mavko et al. 2009):

$$172 \quad C_{ij} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & 0 & 0 & 0 \\ C_{12} & C_{11} & C_{13} & 0 & 0 & 0 \\ C_{13} & C_{13} & C_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & C_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & C_{44} & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{C_{11} - C_{12}}{2} \end{bmatrix}. \quad (4)$$

173 **By solving macroscopic stress and strain in the finite-element analysis, we can estimate all of the elements**
 174 **of macroscopic stiffness based on Eq. (3).** Thus, P-wave velocity V_p and S-wave velocity V_s in the direction
 175 perpendicular to the fracture plane are obtained from

$$176 \quad V_p = \sqrt{\frac{C_{33}}{d}}, \quad V_s = \sqrt{\frac{C_{44}}{d}}, \quad (5)$$

177 where d is the average density of the solid and the fluid (Table 1). The elastic constants of the solid were
178 taken from experimental values; in dry, intact Inada granite under 200 MPa of confining pressure, measured
179 P- and S-wave velocities were 6.14 and 3.42 km/s, respectively.

180 To explore how geophysical properties vary with variations in the fluid distribution within
181 fractures, we investigated the correlations between fracture permeability, flow area, resistivity and elastic
182 wave velocity in detail.

183 3 Results

184 3.1 Changes in Fracture Permeability and Preferential Flow with Aperture Closure

185 Figure 3 shows the three-dimensional fluid flow paths on the smooth and rough fracture surfaces.
186 Flow paths in all models are channelized by asperity contacts (i.e., preferential flow paths). As the fracture
187 aperture closes, both the flow velocity and the number of preferential flow paths decrease. Permeability in
188 each model was calculated from these simulated flow velocities for comparison with the experimental
189 results (Fig. 4). Our digital fracture simulations closely reproduced our experimental results for the smooth
190 (Fig. 4a) and rough (Fig. 4b) surfaces. Plots of the logarithmic permeability against stress show a change
191 with increasing effective normal stress from curving trends to linear trends. Figures 4c and 4d show
192 representative simulation results for the distribution of apertures (in grayscale) and associated flow rates
193 (in color online) through the smooth and rough fractures, respectively. Note that the flows in Fig. 4 represent
194 the vertically summed flow rates (perpendicular to the fracture plane) so that the three-dimensional flows
195 in rough fracture walls can be projected on the x - y plane. These flows are then normalized with respect to
196 their maximum value, and regions with $>1\%$ of the maximum flow rate are visualized to accentuate the
197 dominant flow paths. At low stresses, preferential flow paths form that cover most of the area with open
198 (non-zero) apertures (images i in Fig. 4). Isolated apertures also form, few at first, that are surrounded by
199 contacting asperities (zero aperture points), where the fluid is stagnant (white patches in Fig. 4c, d).

200 As stress increases, larger fractions of the fracture surfaces are in contact, and hence the dominant
201 flow paths decrease in number. As the dominant flow paths become less significant, the flow paths from
202 the inlet to outlet are progressively disconnected (images iii and iv in Fig. 4). Accordingly, the permeability-
203 stress relationship includes a transition: logarithmic permeability changes exponentially with stress while
204 the flow paths are connected (images i and ii) and linearly while the flow paths are disconnected (images
205 iii and iv). The stress level where this change occurs can be defined as the hydraulic percolation threshold
206 σ_{HPT} , which signifies the creation of continuous flow paths through rocks (Guéguen et al. 1997; Kirkby et
207 al. 2016). Roughness does not appear to greatly affect this threshold (see Fig. 5a). Interestingly, the
208 disconnection of dominant electrical flow paths coincides with that of the fluid flow paths (Fig. 12 in

209 Appendix 3), even though electrical flow is spread more diffusely over the fracture than fluid flow (Fig. 13
210 in Appendix 3). Note that both hydraulic and electrical flow do not pass through the matrix owing to its
211 negligibly low permeability and electrical conductivity.

212 3.2 Effect of Stress and Asperity Contact

213 We present the evolution of several rock properties with stress changes in Fig. 5. Note that we
214 discuss only P-wave velocity here as P- and S-wave velocities show similar tendencies (see Fig. 5c).
215 Permeability and resistivity show a linear trend at stresses higher than σ_{HPT} but deviate from a linear trend
216 at lower stresses, and neither property displays any dependence on fracture roughness (Fig. 5a, b). Elastic
217 wave velocity varies notably with roughness, and unlike the case with porous rocks, there is no clear
218 correlation between velocity and porosity; even at the same porosity (for example, ~1.2%), P- and S-wave
219 velocities show variations (Fig. 5c, d).

220 Contact area increases with increasing stress, and hence the hydro-mechanical properties vary
221 likewise (Jaeger et al. 2007; Wang and Cardenas 2016), therefore we examined the effect of contact area
222 on rock properties (Fig. 6). Permeability and resistivity are strongly correlated with contact area and
223 insensitive to roughness (Fig. 6a, b). Previous research has explored the relationship between fracture
224 permeability and contact area in synthetic fractures with identical mean aperture (Zimmerman et al. 1992).
225 Our results, from natural rock fractures with different apertures, also support a stable relationship between
226 permeability and contact area. In contrast, elastic wave velocity is not a single function of contact area,
227 particularly when contact area is larger; instead, velocity generally increases with roughness (Fig. 6c).
228 Although porosity may partially contribute to this velocity variation (Fig. 5c, d), the correlation between
229 them appears to be weak. [Another difference arising from the different roughness characteristics is the size
230 variation of fracture asperity contacts.](#) Figure 7 shows the distribution of contacting asperities along with
231 their size (in color online) in the smooth and rough fractures. Although both fractures have almost the same
232 contact area (~28%), the rough fracture contains larger asperities than the smooth fracture, and the contact
233 area in the smooth fracture consists mostly of small asperities. This difference in spatial distribution of the
234 asperities also produces the velocity difference. The effect of the asperity distribution on the velocity is
235 small when the contact area is low, as contacting asperities in both fracture surfaces are few and small under
236 these conditions.

237 3.3 Relations of Hydraulic and Geophysical Properties

238 We examine the initial hypothesis of the link between hydraulic-electrical-elastic properties in the
239 two plots of Fig. 8. The relationship of P-wave velocity with logarithmic permeability is sensitive to
240 roughness, whereas resistivity clearly shows a simple relationship with permeability on a log-log basis that

241 does not vary with roughness (Fig. 8a). The relationship between logarithmic resistivity and flow area (the
242 areal fraction of preferential flow paths, i.e., the colored areas in Fig. 4c–d) is also insensitive to roughness
243 (Fig. 8b), reflecting the positive correlation between permeability and flow area (Watanabe et al. 2009;
244 Nemoto et al. 2009). The relationship between P-wave velocity and flow area is roughness-dependent when
245 flow areas are below 60% but not so when flow areas exceed 60% (Fig. 8b). This roughness-independent
246 relationship between velocity and flow area at flow areas >60% arises from the roughness independence of
247 velocity in the fracture with lower asperity contacts (Fig. 6). The transition at ~60% flow area coincides
248 with the mechanical percolation threshold, as discussed below.

249 **4 Discussion**

250 **4.1 Effect of Roughness on Rock Properties**

251 We observe that all rock properties change markedly at elevated stresses that increase fracture
252 asperity contacts. Changes in permeability and resistivity with stress (or contact area) are insensitive to
253 roughness, whereas the change in velocity with stress varies notably with roughness. Permeability and
254 resistivity are generally sensitive to pore connectivity (Walsh and Brace 1984; Guéguen and Palciauskas
255 1994), and hence their roughness-independent tendencies may imply that connectivity is unlikely to change
256 with differences in roughness even at the same stress. Although detailed investigations with various samples
257 are needed to assess the correlation of connectivity with these transport properties, the close similarity of
258 the percolation threshold σ_{HPT} in different roughness models also supports our hypothesis. The theoretical
259 study of Zimmerman et al. (1992) shows that transport properties are strongly dependent on the contact
260 area and less sensitive to the microstructure. Because the contact area of different roughness models is
261 almost the same under similar stress conditions in our mated fracture (Table 2), the roughness independence
262 of transport properties in our results may be related to the roughness independence of the contact area. The
263 roughness-independent relationship between resistivity and permeability (Fig. 8) also suggests that at least
264 the mechanisms underlying changes in both properties are the same and do not depend on roughness. Note
265 that such roughness independences may be limited to mated fractures, as the contact area of sheared
266 fractures may be found to change with roughness. The slope of the resistivity-permeability relationship is
267 related to the tortuosity of the pore structure (Brown 1989). The smaller change in resistivity at higher
268 ranges of permeability ($>10^{-11}$) indicates that tortuosity also changes relatively little, whereas the larger
269 change in resistivity at lower permeability ranges ($<10^{-11}$) implies that tortuosity responds dramatically to
270 aperture closure. This change in slope marks a transition of the flow pattern. At higher permeabilities
271 (images i in Fig. 4), flow paths are largely channelized and the flow area is sufficient (>60%), whereas at
272 lower permeabilities (images ii–iv in Fig. 4), flow paths are sinuous (or have fewer connected channels).

273 The roughness dependence of the velocity change arises mainly from differences in porosity and
274 contact area, velocity being higher in samples with lower porosity or larger contact area even at the same
275 stress condition (Figs. 5 and 6). In addition, different roughness characteristics produce size variations of
276 the fracture asperity contacts, which also affect the velocity difference (Fig. 7). On one hand, larger contact
277 sizes generally contribute to stiffening the rock (Guéguen and Boutéca 2004), and hence elastic energy
278 propagates dominantly in the larger asperity due to its high bond energy. On the other hand, a large number
279 of small asperities reduces the bulk elastic stiffness. In the case of cracked materials, thin cracks (i.e.,
280 smaller aspect ratio) reduce the bulk elastic energy more than stiff cracks (i.e., lower aspect ratio) even at
281 the same volume, because the stress strongly concentrates on the edges of thin cracks rather than those of
282 stiff cracks (e.g., Budiansky and O'connell, 1976; Kachanov, 1994). Similarly, our fractured sample also
283 shows a stress concentration on small asperities that are dominant in smooth fractures (Appendix 4).
284 Therefore, we infer that the velocity difference (Fig. 6) may also arise from the size variation of contacting
285 asperities. Figure 9 depicts our conceptual model of roughness-induced variation of asperity contacts and
286 possible changes in velocity. Aperture closure with increasing stress enlarges asperity contacts, and hence
287 the velocity increases in both smooth and rough fractures (Fig. 6). Under higher stress conditions (Fig. 9b
288 and d), even at the same stress and similar proportions of contact area, the asperity size differs due to the
289 roughness, and thus the roughness dependency of velocity is especially marked at higher contact areas. This
290 effect of asperity size is small when the contact area is low, as contacting asperities are few and small under
291 these conditions. Because our results also incorporate the porosity effect, further study is needed to confirm
292 the effect of asperity distribution on velocity by investigations of various natural fractures having identical
293 porosity. It may be of interest that permeability and resistivity do not vary with the size and distribution of
294 asperities because they are integrated properties (Zimmerman et al. 1992), which are insensitive to the
295 microscopic structure but sensitive to the macroscopic structure (i.e., contact area).

296 4.2 Transitions in the Fracture Flow Pattern and Associated Changes in Geophysical Properties

297 Although many experimental studies in intact rocks have revealed the evolution of rock properties
298 with stress change (Brace and Orange 1968; Scholz 2002; Paterson and Wong 2005), some observations
299 have detected unusual changes of rock properties that cannot be explained by these experimental results
300 (Park 1991; Xue et al. 2013). The presence of mesoscale fractures may account for these discrepancies. To
301 investigate this issue, we compiled our results on the evolution of rock properties in single fractures and
302 compared them with the changes in flow rate distribution within the fracture. These changes in rock
303 properties can be categorized as roughness-dependent (Fig. 10a) or roughness-independent (Fig. 10b).

304 Elastic wave velocity and flow area are both roughness-dependent, thus we can distinguish separate
305 mechanical percolation thresholds for smooth fractures (σ_{MPT}) and rough fractures (σ'_{MPT}), defined in both

306 cases as the stress at which velocity reaches 90% of its maximum value (Fig. 10a). Because σ_{MPT} is smaller
307 than σ'_{MPT} , velocity increases more sharply with stress in smooth fractures than in rough fractures. The
308 difference arises from a discrepancy in the heterogeneous aperture distribution (Fig. 1c). In cracked rock
309 samples, it is well known that a rapid velocity increase with stress implies the closure of a dominant set of
310 cracks with a similar aspect ratio (i.e., a sharp bend in the aspect ratio spectrum), whereas a monotonic
311 increase results from closure of cracks of various aspect ratios (i.e., a broader bend in the aspect ratio
312 spectrum) (Tsuji et al. 2008; Mavko et al. 2009). By analogy with this model, a more rapid velocity increase
313 with stress in the smooth fracture may reflect a biased distribution of aperture sizes such that velocity
314 increases rapidly with the closure of apertures of the dominant size and changes only slightly afterward.
315 Resistivity and permeability are both roughness independent (Fig. 10b). Tendencies of these changes
316 depend on the hydraulic percolation threshold σ_{HPT} , which is higher than σ_{MPT} (Guéguen et al. 1997).

317 Figure 9c schematically illustrates these changes in rock properties as three stages (Stage I to Stage
318 III) defined by transitions of the fracture flow pattern within a subsurface fracture with increasing stress.
319 At lower stresses, Stage I represents *aperture-dependent flow*, where fluid flows within most of the void
320 space (the aperture) and the flow area decreases as the mean aperture decreases (Fig. 10a). This stage is
321 typified by largely connected flow paths and sufficient flow area, in which tortuosity is insensitive to stress
322 changes. All rock properties change rapidly with increasing stress in this stage. Stage II, at stresses higher
323 than σ_{MPT} but lower than σ_{HPT} , represents *aperture-independent flow*, in which isolated apertures appear
324 and become areas without flow. In this stage, tortuosity becomes sensitive to stress change, connected
325 channels decrease, and as a result flow area decreases markedly with increasing stress. Unlike Stage I, the
326 rate of decrease in flow area exceeds the decrease in mean aperture size (Fig. 10a), suggesting that the
327 fracture flow at this stage is not fully characterized by aperture size, but instead is controlled by asperity
328 contacts. Although elastic wave velocity remains nearly constant with rising stress, permeability and
329 resistivity change exponentially because flow paths are still connected and thus these attributes are less
330 sensitive to the spatial distribution of asperity contacts. In Stage III, at stresses higher than σ_{HPT} , flow paths
331 become disconnected and result in *disconnected flow*. In this stage, logarithmic permeability and resistivity
332 change linearly with stress, and areas without flow become a significant fraction of the fracture area.
333 Because Stage II begins when the velocity ceases to change with rising stress, the transition from Stage I
334 to II can be detected by velocity monitoring, whereas resistivity is sensitive to the transition from Stage II
335 to III. This means that, if monitoring detects the combination of almost constant velocity and exponential
336 change in the logarithmic resistivity, it may signal the presence of aperture-independent (Stage II) flow.

337 If crustal stress can be considered constant (i.e., on relatively short timescales), then changes in the
338 fracture flow pattern with changes in effective normal stress represent changes in pore pressure. This

339 finding may show promise in two applications. One application involves the evolution of fluid flow along
340 faults, which is part of the fault reactivation cycle triggered by pore pressure perturbations. Our model of
341 Stage I reproduces observations of high permeability (Xue et al. 2013; Kinoshita et al. 2015), low resistivity
342 (Mazzella and Morrison 1974; Park 1991) and low seismic velocity (Brenquier et al. 2008; Taira et al.
343 2018) resulting from high pore pressures associated with earthquakes. The changes in elastic wave velocity
344 and permeability from Stage I to II (Fig. 10a, b) are in good agreement with observations after earthquakes
345 (Xue et al. 2013; Nimiya et al. 2017). Under Stage II conditions, a resistivity change of ~10–20% (Park
346 1991) corresponds to a stress perturbation of 0.2–1.4 MPa, and a permeability change of ~30–40% (Xue et
347 al. 2013) corresponds to a stress perturbation of 0.9–3.2 MPa. Moreover, during Stage II, seismic velocity
348 is nearly constant after healing stabilizes the mechanical properties of faults (Nimiya et al. 2017).
349 Nevertheless, subsurface fracture flow could be changing because our results show that seismic velocity is
350 insensitive to pressure above σ_{MPT} . Fault healing eventually leads to large areas of little or no flow (Stages
351 II and III), where mineral precipitation is favored. Pore pressure changes following earthquakes, triggered
352 by several mechanisms such as mineral precipitation (Sibson 1992; Tenthorey et al. 2003), lead rapidly to
353 decreases in seismic velocity, increases in permeability and decreases in resistivity, after which all of these
354 properties recover (Mazzella and Morrison 1974; Xue et al. 2013; Taira et al. 2018), which suggests that
355 fracture flow patterns return to their initial condition (Stage I). Thus our inferred transitions in the fracture
356 flow pattern may explain how the cycle of earthquake recurrence is correlated with geophysical
357 observations, complementing the fault-valve model (Sibson et al. 1988).

358 The other application involves the changes in productivity of fluid resources in fractured reservoirs
359 (for example, geothermal reservoirs) during development. Because increased elastic wave velocity
360 coincides with decreased permeability during Stage I, a gradual velocity increase in geothermal fields
361 implies a slight decrease in reservoir permeability (Taira et al. 2018). If a point is reached where velocity
362 remains steady while resistivity decreases, the fracture flow pattern would be at Stage II or III, where the
363 flow area shrinks considerably. A limited flow area could lead to poorer thermal performance during a
364 geothermal development (Hawkins et al. 2018) and could lower reservoir permeability by as much as two
365 orders of magnitude (Fig. 9).

366 To apply our results to real field locations, we need to consider the scale dependencies of rock
367 properties. For example, although longer fracture lengths generally mean higher roughness values (Brown
368 and Scholz 1985; Power et al. 1987; Power and Durham 1997; Jaeger et al. 2007), fracture permeability in
369 joints is only partially dependent on fracture length (Ishibashi et al. 2015). This suggests that roughness-
370 independent properties, including resistivity (Fig. 10b), may have a weak dependence on fracture length,
371 thus resistivity monitoring could be effective for detecting changes in hydraulic properties at field scale.

372 On the other hand, elastic wave velocity is a roughness-dependent property (Fig. 10a) and thus varies with
373 the fracture scale. However, this scaling effect on velocity can be modified by considering the ratio of the
374 wavelength and the fracture length (Mavko et al. 2009). Although our study adopted a zero-frequency
375 assumption for the velocity calculation, the scaling effect on velocity can be addressed by considering finite
376 wavelengths. Because finite-difference time-domain modeling of wave fields in fractured media requires
377 more complex assumptions, such as fracture compliance (Bakulin et al. 2000; Minato and Ghose 2016;
378 Pyrak-Nolte et al. 1990), the scale dependency on velocity needs to be further explored.

379 **4 Conclusions**

380 We investigated the correlated changes in fracture permeability, flow area, resistivity and elastic
381 wave velocity of joints under increasing normal stress by coupling experimental data with digital fracture
382 simulations. We found that changes in permeability and resistivity are controlled by fluid connectivity,
383 which is more dependent on stress than on fracture roughness. The relationship between hydraulic and
384 electrical properties is independent of roughness, owing to the roughness independence of fluid connectivity
385 (as expressed by the hydraulic percolation threshold). The roughness dependence of elastic wave velocity
386 arises from spatial distributions of contacting asperities as well as the roughness dependency of porosity.
387 These relationships show promise for improving geophysical interpretations. Our lattice Boltzmann fluid
388 flow simulation revealed that the fracture flow pattern undergoes transitions through three stages as
389 effective normal stress increases: aperture-dependent flow (Stage I), aperture-independent flow (Stage II)
390 and disconnected flow (Stage III). Elastic wave velocity may be a useful indicator of the Stage I–II
391 transition, and resistivity may be a sensitive indicator of the Stage II–III transition. The relationships we
392 have revealed may enable geological regimes associated with stress changes, such as seismogenic zones
393 and geothermal reservoirs, to be monitored remotely on the basis of their geophysical properties.

394

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405 simulation results are summarized in an appendix. The digital fracture data are available online from
406 <http://geothermics.mine.kyushu-u.ac.jp/sawayama/rmre2020> and from the Digital Rocks Portal
407 (<http://www.digitalrocksportal.org/projects/273>).

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578

579 **Figures and Tables**

580 **Table 1.** Physical properties used for finite-element modeling of resistivity and elastic wave velocity.

581

	Conductivity [mS/m]	Bulk modulus [GPa]	Shear modulus [GPa]	Density [kg/m ³]
Solid	0.01*	58.6***	31.0***	2650
Fluid	5000**	2.25	0	994

582

* Based on the experimental result of the resistivity measurement under dry condition

583

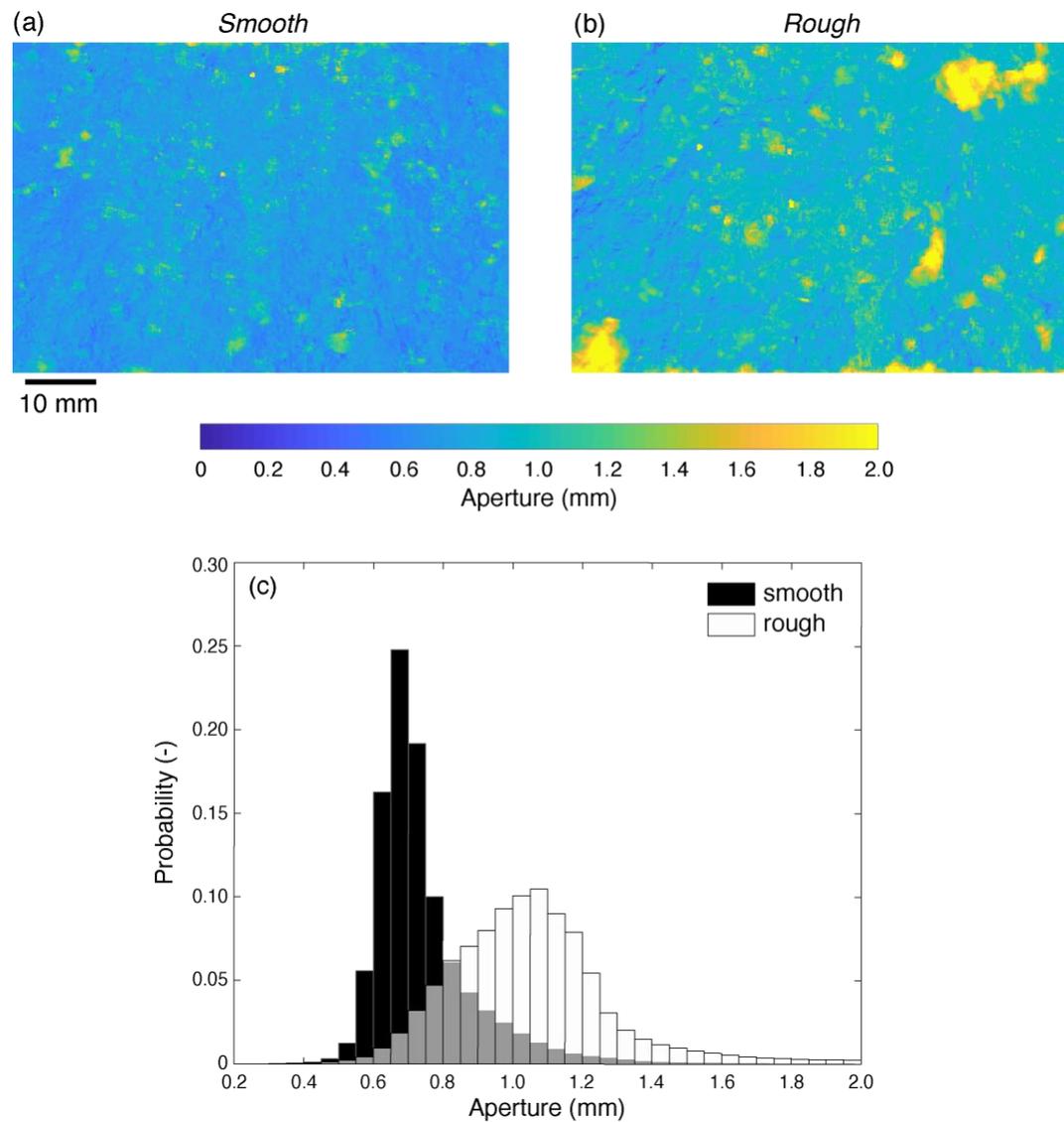
** For seawater

584

*** Based on P- and S-wave velocity measurements under dry conditions and high confining pressure (200 MPa)

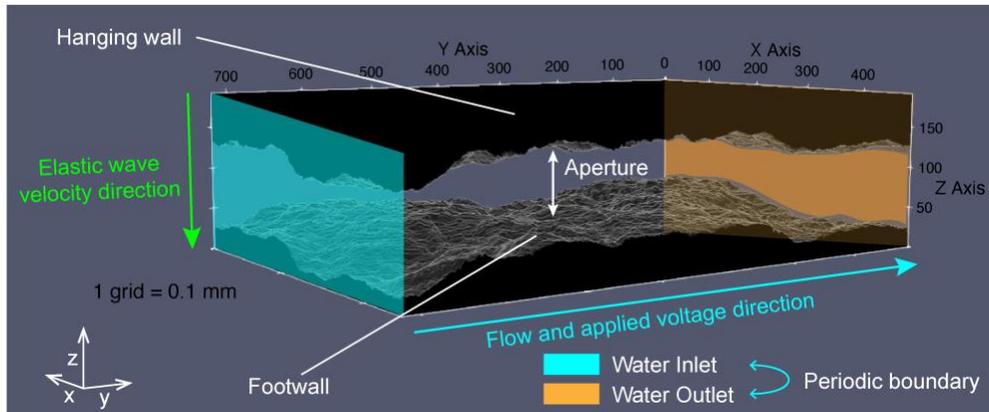
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587

588 **Fig. 1** Fracture aperture distribution of (a) smooth and (b) rough fracture and (c) probability histogram of
 589 apertures. Color in (a) and (b) represents the fracture aperture (color figure online)

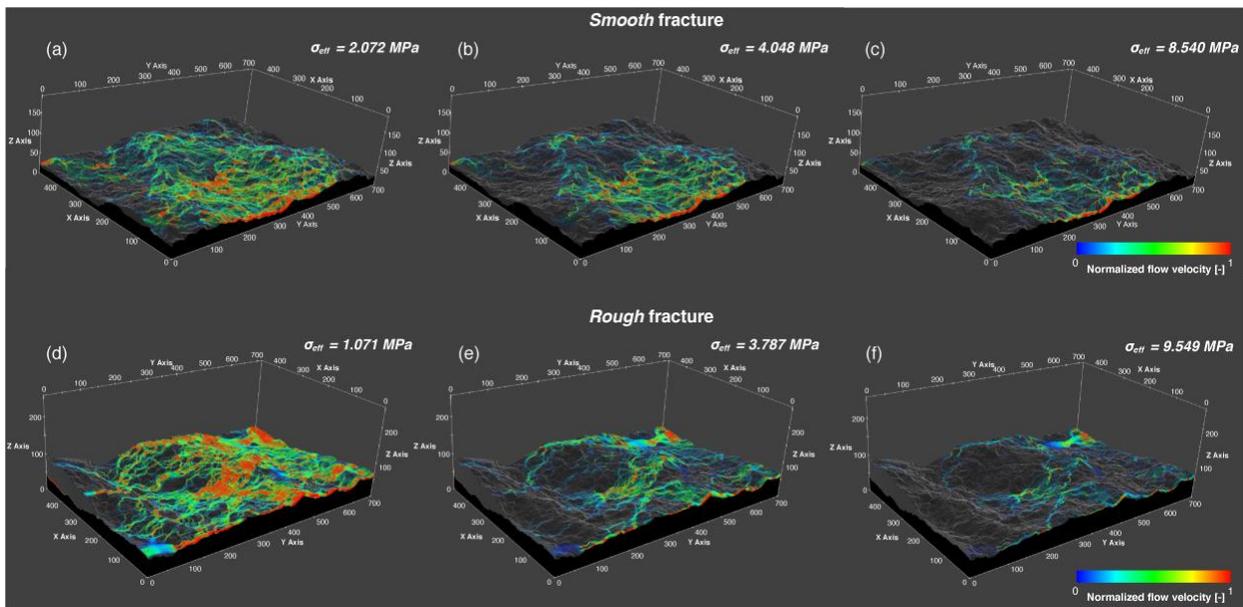


590

591 **Fig. 2** Model setup of the 3D digital fracture simulation. Fluid flow and applied voltage are defined as
 592 parallel to the fracture plane, whereas elastic wave velocity is defined as perpendicular to the fracture plane.
 593 Both the lattice Boltzmann simulation and finite-element modeling adopt a periodic boundary condition

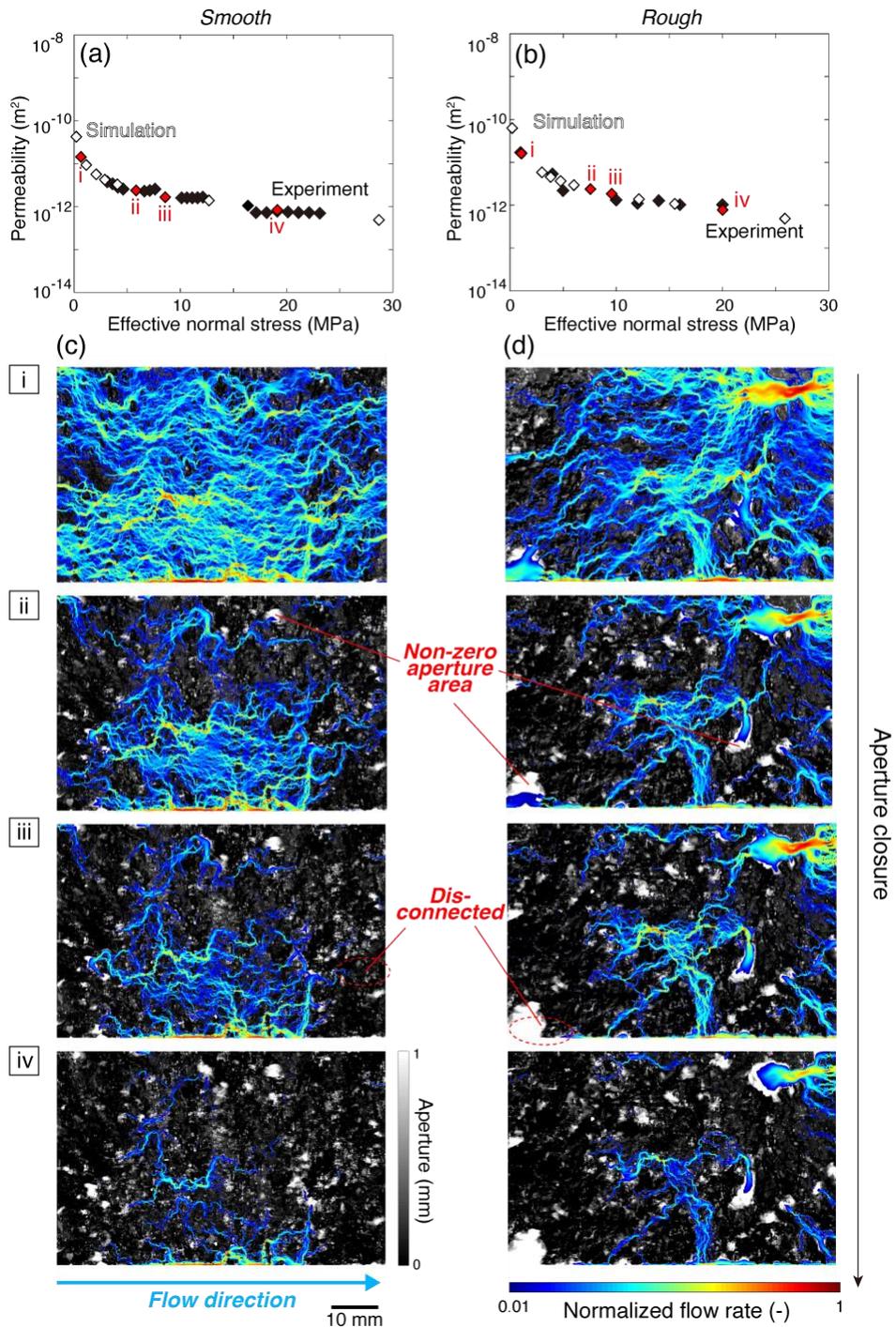
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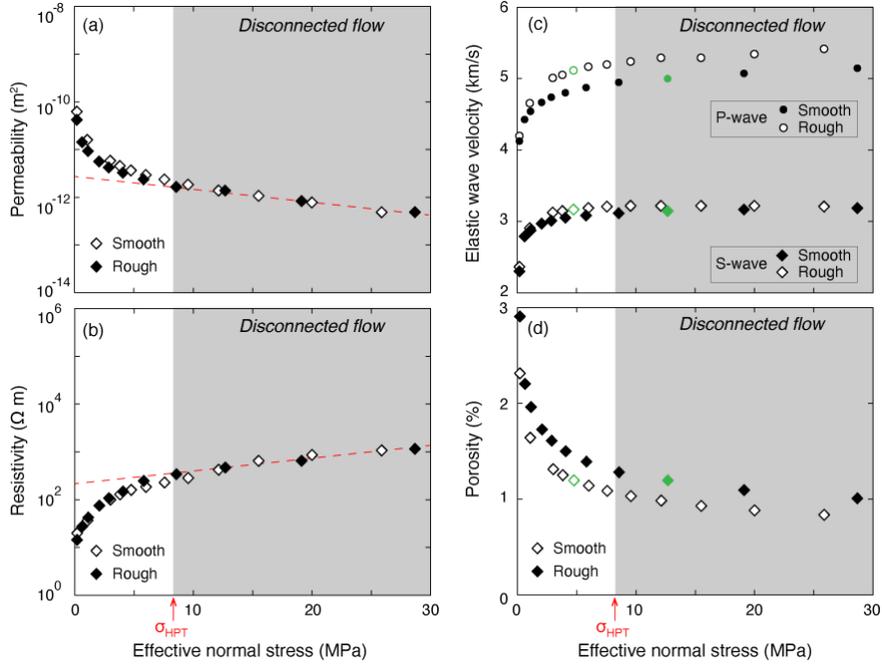


596

597 **Fig. 3** Three-dimensional flow paths calculated by the lattice Boltzmann simulation on the surface of the
 598 (a–c) smooth and (d–f) rough fracture under various effective normal stress (σ_{eff}). Flow velocity (in color
 599 online) is illustrated on the footwall of each fracture surface

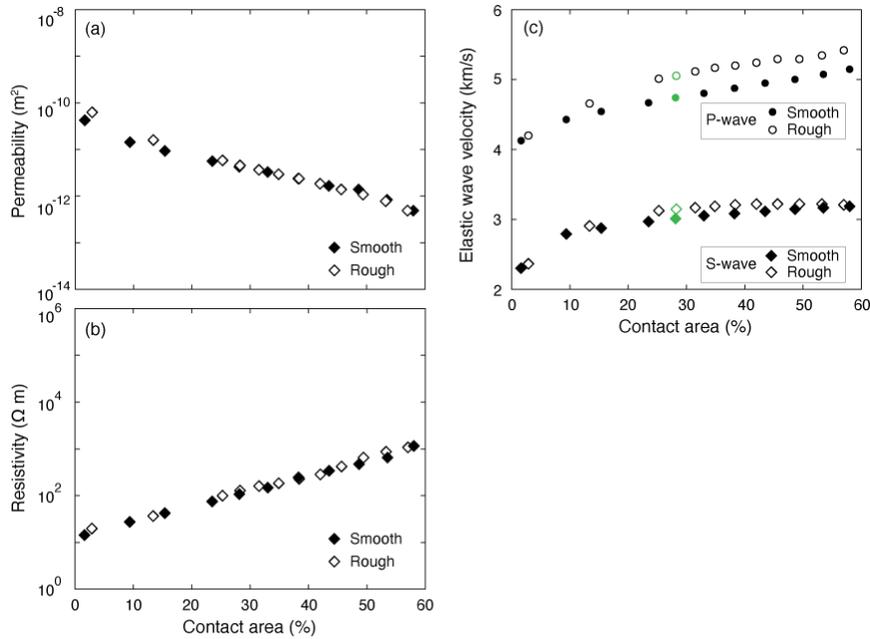


600
 601 **Fig. 4** Experimental and simulated fracture permeabilities with increasing effective normal stress of the (a)
 602 smooth and (b) rough fractures and representative images derived from the simulation showing fracture
 603 flow distribution (color) within the heterogeneous aperture distribution (grayscale) with aperture closure of
 604 the (c) smooth and (d) rough fractures. Black and white diamonds in (a) and (b) represent experimental and
 605 simulated results, respectively. Red diamonds in (a) and (b) are the representative results that are illustrated
 606 in (c) and (d). The normalized flow in (c) and (d) represents the vertical summed flow, normalized by the
 607 maximum value in each condition, and the regions with <1% of the maximum flow rate are colorless (color
 608 figure online). Dashed red ellipses in (c) and (d) show regions that are disconnected from the dominant flow
 609 paths



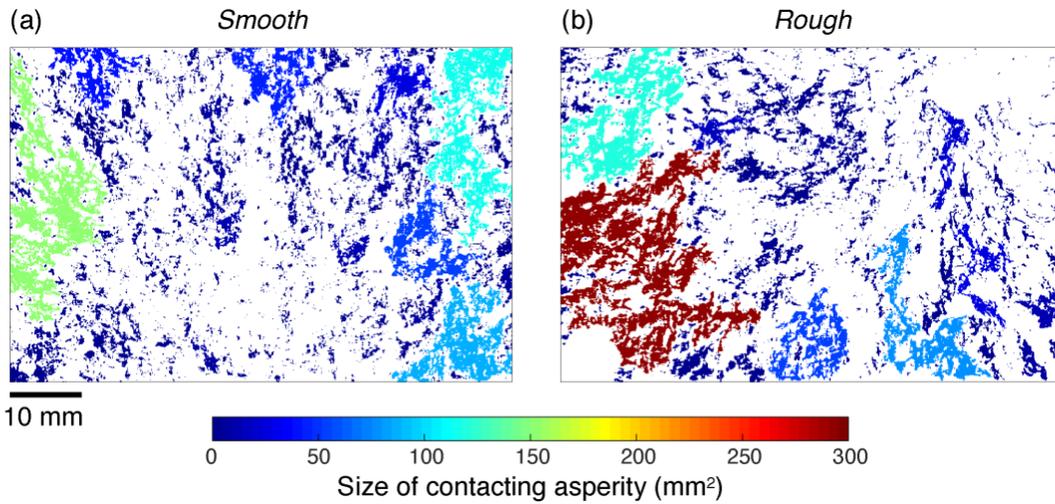
610

611 **Fig. 5** Graphs showing changes in (a) permeability, (b) resistivity, (c) elastic wave velocity and (d) porosity
 612 in relation to effective normal stress. Dashed lines are extrapolations from the data in the regions of
 613 disconnected flow (gray), as defined by the value of σ_{HPT} . Gray symbols in (c) and (d) (green in the online
 614 version) represent pairs of data points that have comparable porosity (~1.2%)



615

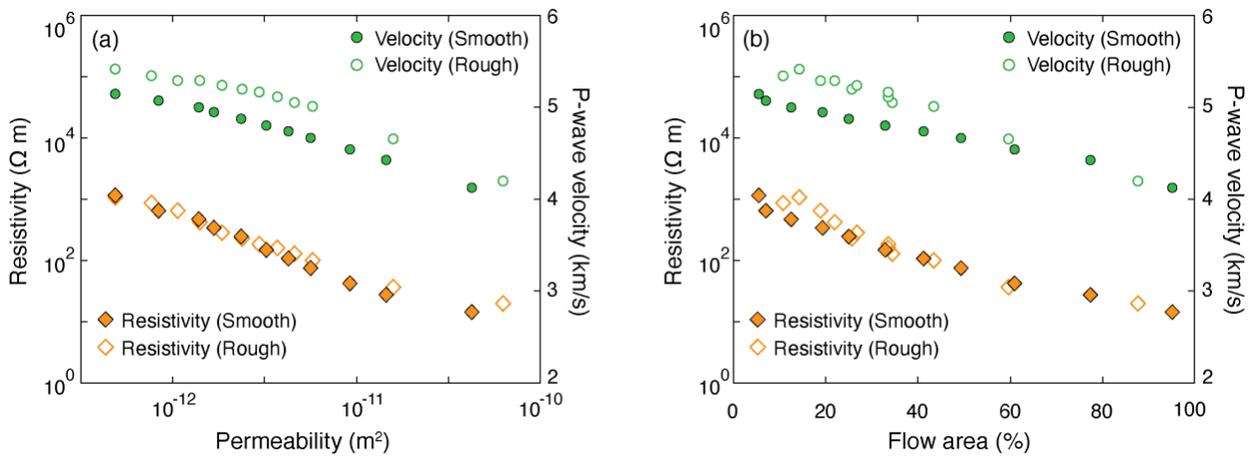
616 **Fig. 6** Graphs showing changes in (a) permeability, (b) resistivity, and (c) elastic wave velocity in relation
 617 to the contact area. Gray symbols in (c) (green in the online version) represent pairs of data points with
 618 comparable contact area (~28%), whose asperity distributions are shown in Fig. 7



619

620 **Fig. 7** Distribution of asperity contacts on the (a) smooth and (b) rough fractures, both of which have a
 621 contact area of ~28%. Color represents the asperity size (color figure online)

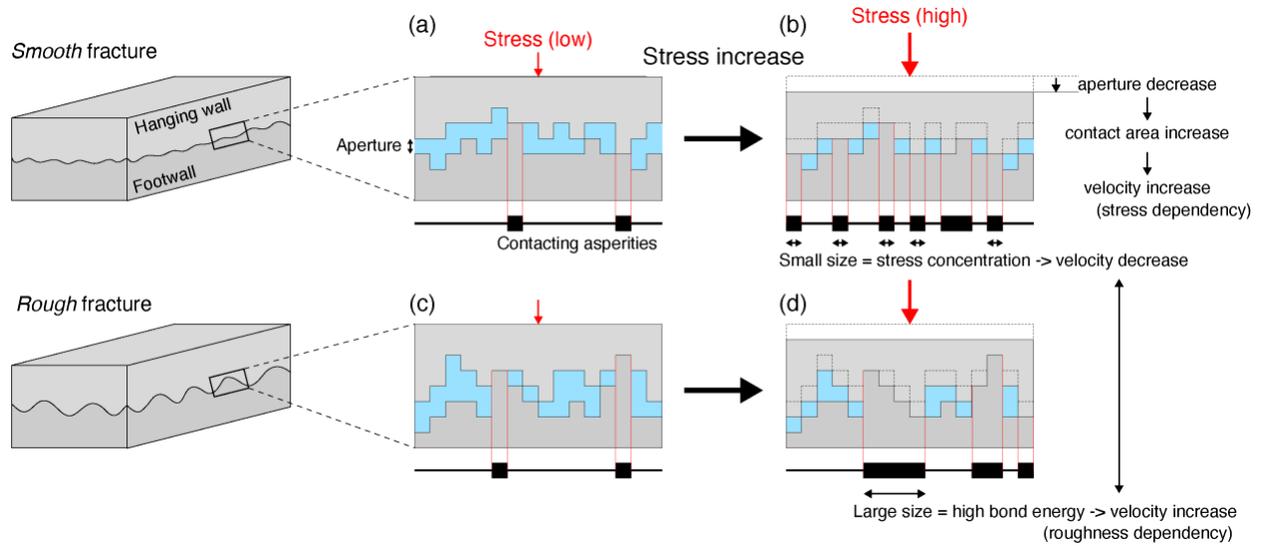
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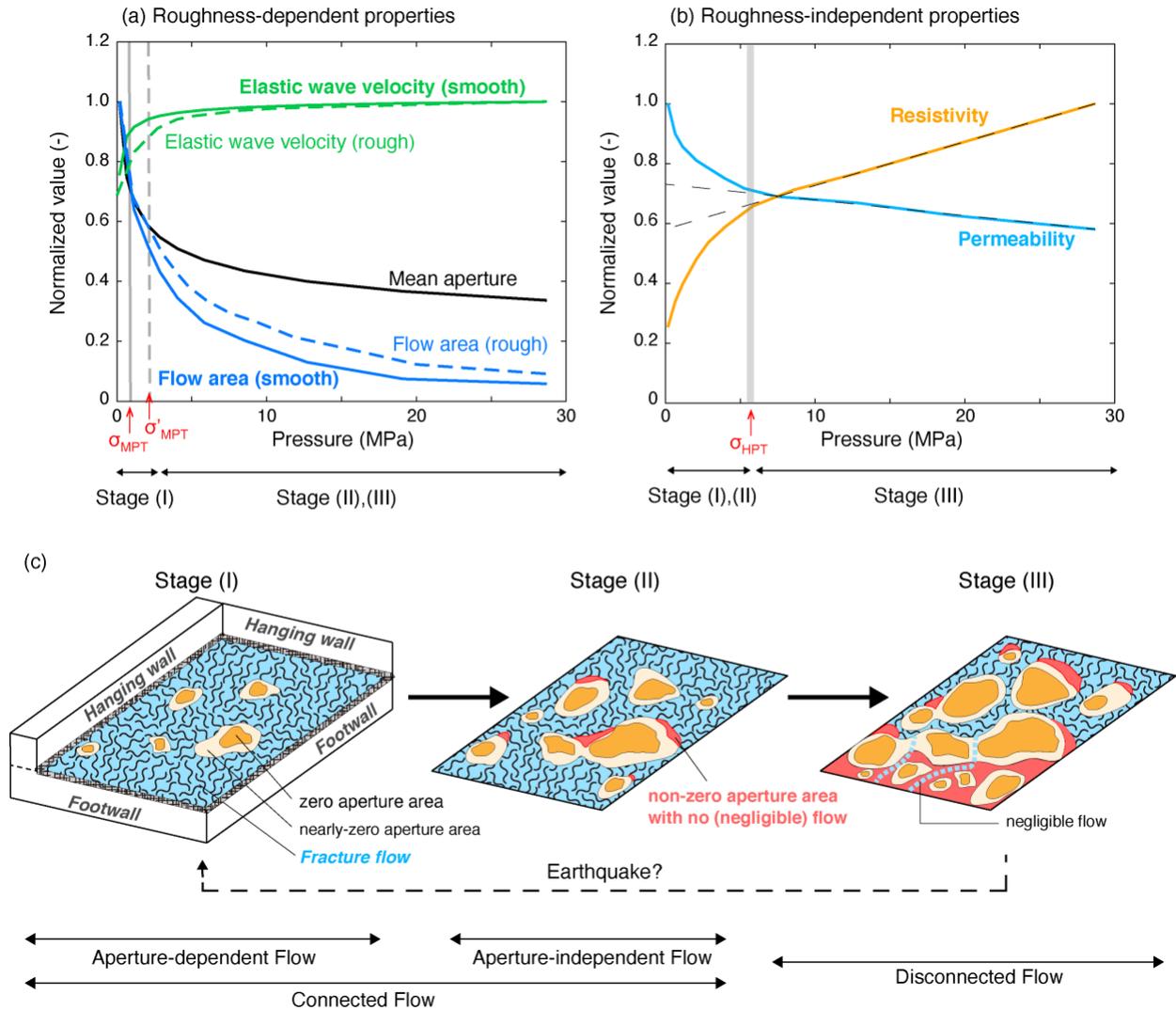
624 **Fig. 8** Graphs showing correlations between (a) permeability and geophysical properties and (b) flow area
 625 and geophysical properties. Orange diamonds and green circles (color online) represent resistivity and P-
 626 wave velocity, respectively, and open and solid symbols represent smooth and rough fractures, respectively

627



628

629 **Fig. 9** Schematic images of the voxel model of the fracture aperture structure and asperity contacts, showing
 630 their changes with stress for (a–b) smooth and (c–d) rough fractures. The apertures are shown in blue,
 631 matrix in gray, and contacting asperities as black solid boxes (color figure online)

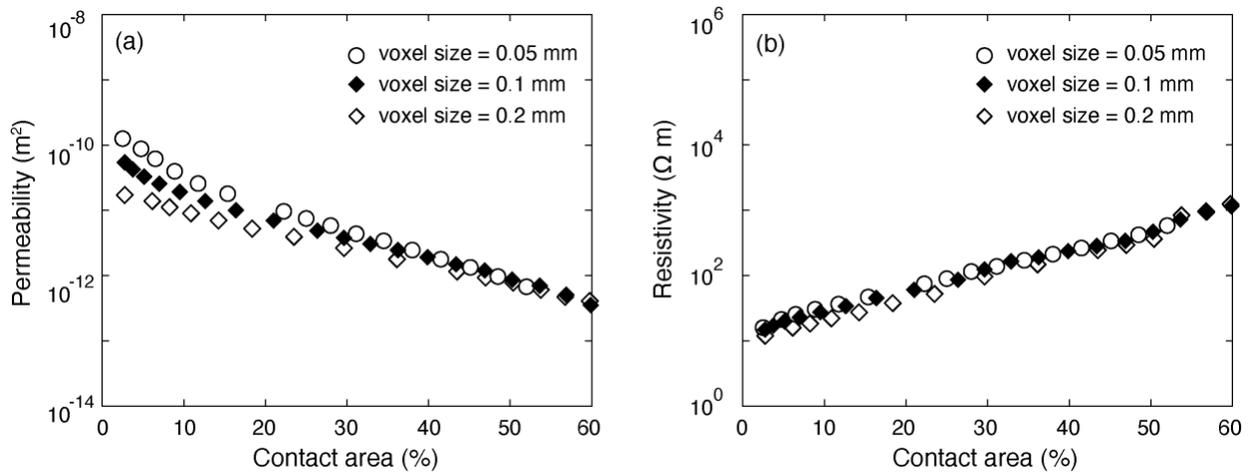


632

633 **Fig. 10** Schematic diagram of changes with respect to pressure in (a) roughness-dependent properties and
 634 (b) roughness-independent properties and (c) schematic images of the three-stage transition of fracture flow
 635 patterns. All rock physical properties in (a) and (b) are normalized based on our results. Gray lines in (a)
 636 represent mechanical percolation thresholds σ_{MPT} and σ'_{MPT} of smooth and rough fractures, respectively,
 637 which distinguish aperture-dependent and aperture-independent flows (Stages I and II). The gray line in (b)
 638 represents the hydraulic percolation threshold σ_{HPT} , which represents the boundary between connected
 639 flow (Stages I and II) and disconnected flow (Stage III)
 640

641 **Appendix 1. Effect of Voxel Size**

642 The voxel size potentially affects the absolute value of permeability and resistivity because these quantities
643 are sensitive to the connectivity of the local aperture. To check this possible effect of voxel size, we
644 analyzed the permeability and resistivity of models with different voxel sizes, preparing $48\text{ mm} \times 48\text{ mm}$
645 fracture models from the rough fracture surfaces using cubic systems with 0.05 mm, 0.1 mm, and 0.2 mm
646 voxels. Figure 11 plots the permeability and resistivity against the contact area from the models of each
647 voxel size. Although voxel size affects permeability to some degree, the maximum difference between the
648 results with 0.05 mm and 0.1 mm voxels is less than half an order of magnitude (Fig. 11a). The difference
649 in resistivity is much smaller (Fig. 11b). Notably, the models with 0.05 mm and 0.1 mm voxel sizes show
650 similar trends in both cases of permeability and resistivity. Because the computational cost is prohibitive at
651 our original fracture size ($48\text{ mm} \times 72\text{ mm}$) in a 0.1 mm cubic system, we conclude that the 0.1 mm voxel
652 size is suitable for our qualitative interpretations of permeability and resistivity.



656 **Fig. 11** Graphs showing (a) permeability and (b) resistivity with different sizes of voxel. Open diamonds,
657 solid diamonds, and open circles represent the results from 0.2, 0.1, and 0.05 mm voxel sizes, respectively

658

659 **Appendix 2. Supplementary Material**

660 Table 2 summarizes the simulation results. Movie files of the lattice Boltzmann simulations can be found
 661 online.

662
 663

664 **Table 2.** Summary of simulation results: σ_{eff} is the effective normal stress, ϕ is the porosity, k_f is the fracture
 665 permeability, and ρ is the electrical resistivity. The smooth fracture at $\sigma_{eff} = 12.7$ MPa and the rough fracture
 666 at $\sigma_{eff} = 4.76$ MPa have similar porosity (1.19%), but differ in velocity (V_p) by 0.11 km/s. Similarly, the
 667 smooth fracture at $\sigma_{eff} = 2.87$ MPa and the rough fracture at $\sigma_{eff} = 3.79$ MPa have similar contact area (~28%),
 668 but differ in velocity (V_p) by 0.32 km/s

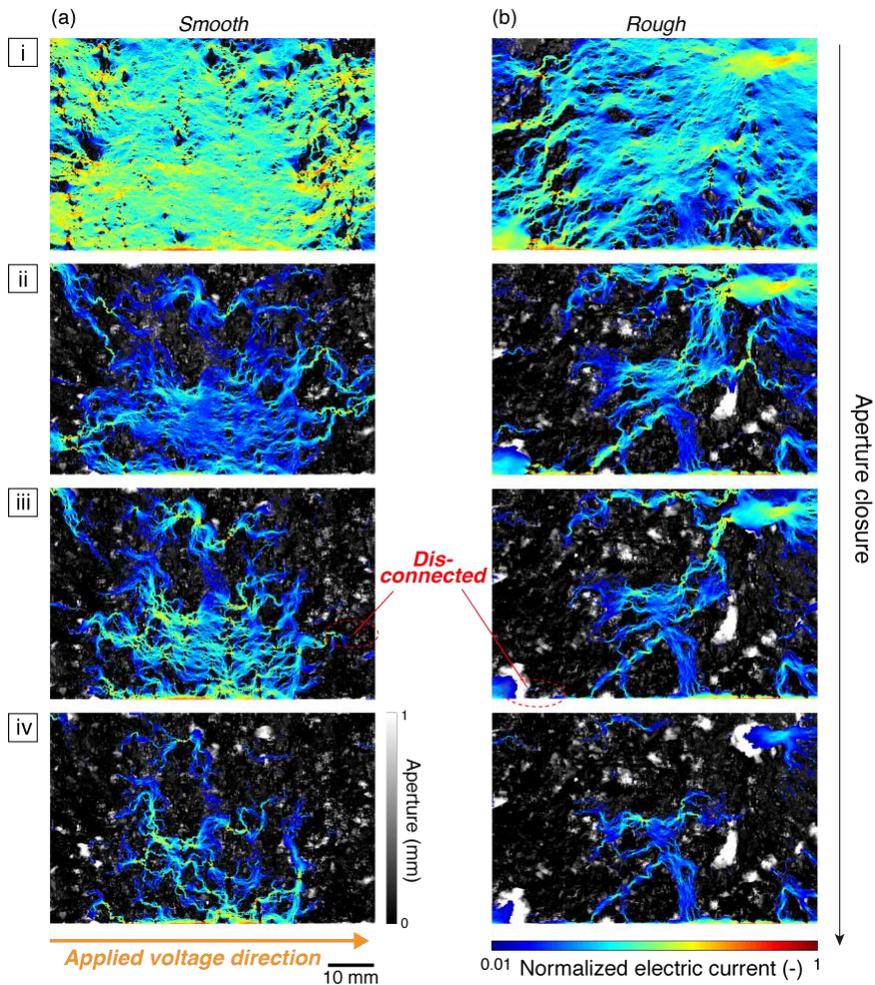
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σ_{eff} [MPa]	Mean aperture [mm]	Contact area [%]	ϕ [%]	$\log_{10}k_f$ [m ²]	Flow area [%]	$\log_{10}\rho$ [Ω m]	V_p [km/s]	V_s [km/s]
<i>Smooth fracture</i>								
0.165	0.238	1.66	2.91	-10.4	95.3	1.16	4.12	2.30
0.654	0.178	9.41	2.20	-10.8	77.4	1.44	4.42	2.79
1.14	0.158	15.4	1.96	-11.0	61.0	1.63	4.54	2.88
2.07	0.139	23.4	1.73	-11.3	49.4	1.88	4.67	2.97
2.87	0.130	28.1	1.61	-11.4	41.2	2.04	4.74	3.01
4.05	0.121	33.0	1.50	-11.5	32.8	2.18	4.80	3.05
5.83	0.112	38.2	1.39	-11.6	25.0	2.39	4.87	3.09
8.54	0.103	43.5	1.28	-11.8	19.3	2.53	4.95	3.12
12.7	0.0951	48.6	1.19	-11.9	12.4	2.67	5.01	3.15
19.1	0.0873	53.5	1.10	-12.1	7.12	2.82	5.08	3.17
28.7	0.0801	58.0	1.01	-12.3	5.57	3.06	5.14	3.19
<i>Rough fracture</i>								
0.208	0.330	2.91	2.32	-10.2	87.6	1.30	4.20	2.37
1.07	0.233	13.4	1.64	-10.8	59.8	1.57	4.66	2.91
3.02	0.187	25.2	1.32	-11.2	43.3	2.00	5.01	3.12
3.79	0.178	28.3	1.25	-11.3	34.4	2.11	5.06	3.15
4.76	0.169	31.5	1.19	-11.4	33.6	2.20	5.12	3.17
6.00	0.161	34.9	1.14	-11.5	33.6	2.27	5.16	3.19
7.55	0.154	38.4	1.09	-11.6	25.8	2.36	5.20	3.21
9.55	0.146	42.0	1.04	-11.7	26.6	2.45	5.24	3.22
12.1	0.139	45.6	0.984	-11.9	22.0	2.62	5.29	3.22
15.5	0.132	49.4	0.933	-12.0	18.8	2.81	5.29	3.22
20.0	0.125	53.2	0.884	-12.1	10.8	2.94	5.34	3.22
25.9	0.118	57.0	0.836	-12.3	14.2	3.03	5.41	3.21

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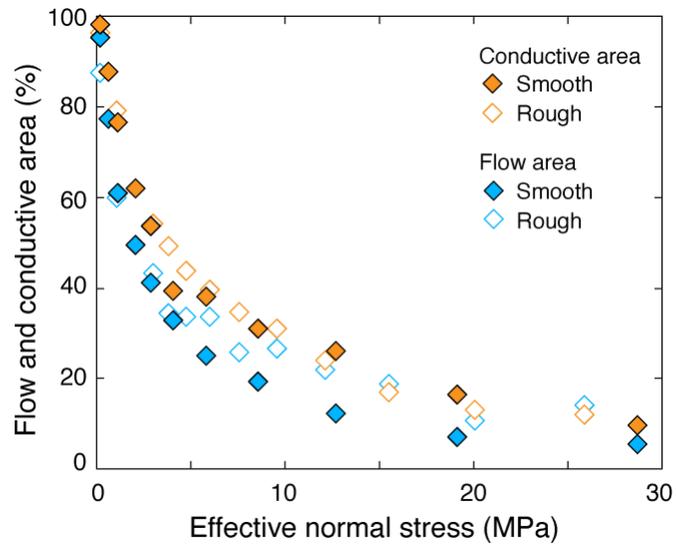
673 **Appendix 3. Local Electrical Flow**

674 The local electrical flows are visualized in Fig. 12 in the same fashion as the fluid flow paths in Fig. 4 in
 675 the main text. The flow in Fig. 12 shows vertically summed electric currents (perpendicular to the fracture
 676 plane), normalized with respect to their maximum value. Regions with >1% of the maximum electric
 677 current are visualized to accentuate the dominant paths. Although the trend of transient changes of electrical
 678 flow with aperture closure is similar to that of fluid flow, electrical flow is spread more diffusely over the
 679 fracture than fluid flow (Brown 1989). From these results, the conductive area is calculated, defined as the
 680 ratio of the area of dominant electrical flow paths to the area of the fracture plane (colored area in Fig. 12).
 681 Figure 13, which plots the evolution of both the conductive area and flow area at elevated stress, clearly
 682 shows that conductive area is slightly greater than flow area in both smooth and rough fractures. It is notable
 683 that the disconnection of dominant electrical flow paths coincides with that of the fluid flow paths (i.e.,
 684 hydraulic percolation threshold).
 685



686

687 **Fig. 12** Local electrical flow distribution (color) within the heterogeneous aperture distribution (grayscale)
 688 with aperture closure of the (a) smooth and (b) rough fractures. Images i–iv are representative results at the
 689 same stress conditions as in Fig. 4. The normalized electric current represents the vertical summed electric
 690 current, normalized by the maximum value in each condition, and the regions with <1% of the maximum
 691 electric current are colorless (color figure online). Dashed red ellipse shows a regions that are disconnected
 692 from the dominant paths



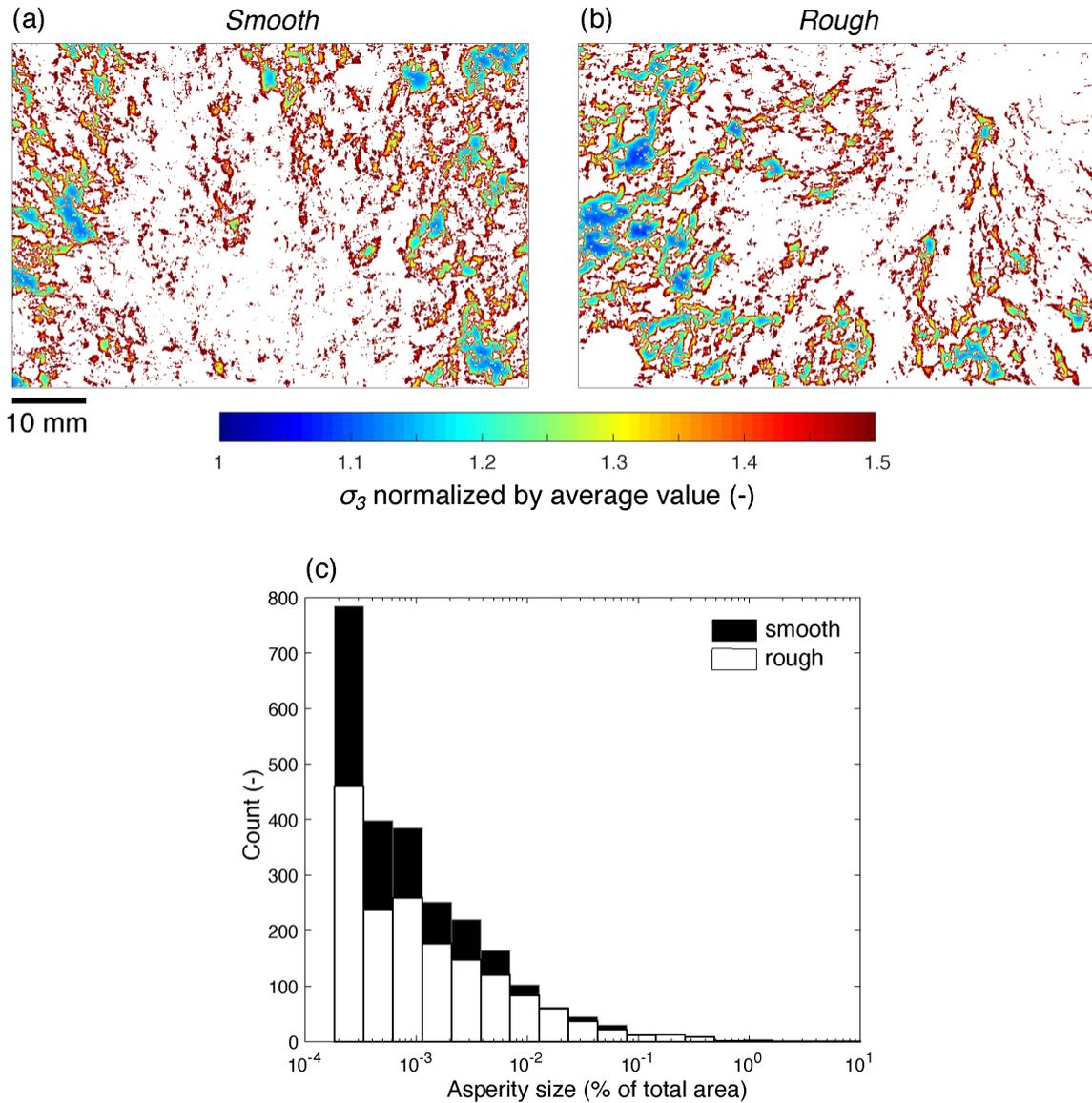
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694 **Fig. 13** Graphs showing changes in flow area (blue symbol) and conductive area (orange symbol) in relation
 695 to effective normal stress. Open and closed diamonds show the results from smooth and rough fractures,
 696 respectively

697

698 **Appendix 4. Stress Concentration on Small Asperities**

699 To reveal the effect of asperity size on the stress concentration, we visualized the local distribution of stress
 700 perpendicular to the fracture plane (σ_3). Figures 14 shows the distribution of σ_3 at the same condition as
 701 Fig. 7 (contact area ~28%) in smooth and rough fractures. The stress value is normalized by its average and
 702 visualized only in asperities. In both cases, the stress concentrates strongly on smaller asperities, whereas
 703 the stress across larger asperities is relatively small. Smaller asperities are more dominant in the smooth
 704 fracture case (Fig. 14c).
 705



706
 707 **Fig. 14** Local distribution of stress (color) across the fracture plane in contact areas at the same condition
 708 as Fig. 7 (contact area ~28%) of the (a) smooth and (b) rough fractures and (c) histogram of asperity size
 709 in the smooth (black) and rough (white) fractures (color figure online)