Salinity Management in the World's Most Saline Dam Reservoir: The Gotvand Reservoir, Iran

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October 19, 2023

Abstract

The Gotvand dam was built on the most important Iranian river to support a number of populated cities with freshwater, provide irrigation water for million hectares of fertile farmlands, and meet water demand for the country's hub industrial zones. This dam is known as one of the worst engineering failures in Iran's history because its impoundment submerged the enormous salty unit of Gachsaran evaporite formation (GEF) outcropped in the reservoir, leading to reservoir water salinization in deep layers up to several times greater than that of in the high-seas. Given the failed practical application of direct intervention strategies to control the salinity crisis, we suggested a low-cost salinity management strategy based on the reservoir operation to mitigate the dam outlet salinity and preserve the downstream environment from the salinity hazards. The three-dimensional MIKE3 model, was run to calculate the GEF dissolution rate, accumulated salt in the reservoir, and the dam outlet salinity. Then, we ran the model considering different outlet salinity for downstream irrigation-use. Simulation results suggested that the GEF dissolution rate varied from 0.5 to 7 cm/hr, mainly due to incremental submergence of the GEF during multi-stage impoundment of the reservoir. Considering the final dissolution rate of 0.5 cm/hr and inlet salinity from the upstreams, salt accumulation inside the reservoir can be gradually prevented by setting the outlet salinity to its maximum historical downstream level, i.e., 1400 µmhos/cm.

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Salinity Management in the World's Most Saline Dam Reservoir: The Gotvand Reservoir, 1 2 Iran

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14 **Key Points:**

- A modeling approach was used to manage salinity in the world's most saline dam reservoir, 15 i.e., the Gotvand reservoir, Iran. 16
- 17 • Pycnocline established in the early impoundment stage lasted during the study period, leading to a crenogenic meromixis in the reservoir. 18
- Findings show that reservoir salinity management could mitigate outlet salinity and preserve 19 20 downstream environment from salinity hazards.

21 Abstract

22 The Gotvand dam was built on the most important Iranian river to support a number of populated 23 cities with freshwater, provide irrigation water for million hectares of fertile farmlands, and meet 24 water demand for the country's hub industrial zones. This dam is known as one of the worst engineering failures in Iran's history because its impoundment submerged the enormous salty unit 25 of Gachsaran evaporite formation (GEF) outcropped in the reservoir, leading to reservoir water 26 27 salinization in deep layers up to several times greater than that of in the high-seas. Given the failed 28 practical application of direct intervention strategies to control the salinity crisis, we suggested a 29 low-cost salinity management strategy based on the reservoir operation to mitigate the dam outlet 30 salinity and preserve the downstream environment from the salinity hazards. The three-dimensional 31 MIKE3 model, was run to calculate the GEF dissolution rate, accumulated salt in the reservoir, and 32 the dam outlet salinity. Then, we ran the model considering different outlet salinity levels to explore 33 the best reservoir operation strategy to prohibit the accumulated salt in the reservoir and keep the 34 safe salinity for downstream irrigation-use. Simulation results suggested that the GEF dissolution 35 rate varied from 0.5 to 7 cm/hr, mainly due to incremental submergence of the GEF during multi-36 stage impoundment of the reservoir. Considering the final dissolution rate of 0.5 cm/hr and inlet 37 salinity from the upstreams, salt accumulation inside the reservoir can be gradually prevented by 38 setting the outlet salinity to its maximum historical downstream level, i.e., 1400 µmhos/cm.

39 1 Introduction

40 Dam construction has always been a major engineering solution in both industrialized and developing countries to mitigate hydrologic hazards, supply anthropogenic water demands, and 41 42 promote human welfare and development (Simonovic and Arunkumar, 2016; Best, 2019; Winton et al., 2019; Mulligan, 2020). However, many of these hydro systems have encountered environmental 43 44 and economic failure due to, among other things, eutrophication (Zaragüeta and Acebes, 2017; 45 Noori et al., 2021), sedimentation (Wang and Kondolf, 2014), and reservoir water salinization 46 (Kerachian and Karamouz, 2007; Tavoosi et al., 2022). Reservoir water salinization is mainly 47 associated with the submerged salty unit of evaporite formations and saline tributaries (Jalali et al., 48 2019). Outcropped saline geological formations in reservoir areas have critical implications for reservoir water quality, dam structures, and the downstream ecosystems, which may prohibit the 49 50 dams from (in some cases) no longer being able to fulfill their purposes (Poff et al., 2016; Winton et 51 al., 2019). Therefore, reservoir salinity management is required to mitigate the negative impacts of 52 water quality and potential downstream environmental consequences.

53 The Gotvand dam, the greatest mistake in the history of Iranian engineering, was built on 54 the country's most important river, i.e., Karun River, in a region with an outcropped salty unit of 55 geological formations (mainly, Gachsaran evaporite formation – GEF) in 2011. Due to the Gotvand 56 reservoir impoundment, vast GEF within the reservoir area was submerged, making the reservoir 57 water increasingly saline. The salinity of the reservoir water in the bottom layers has reached up to several times higher than the salinity of the high-seas (Aghasian et al., 2019), establishing the 58 59 Gotvand as the world's most saline drinking water reservoir (based on our knowledge). Therefore, 60 if adequate and necessary measures are not considered to control the reservoir water outlet 61 immediately, salinization would threaten the dam reservoir and the downstream environment that consists of million hectares of fertile agricultural lands, Iran's hub industrial zones, populated cities 62 and unique protected areas (Naderkhanloo, 2013; Fakouri et al., 2019; Malek Mohammadi et al., 63 64 2022). In the face of the present problem, different managerial strategies were outlined, as 65 summarized in Table S1. These strategies can be divided into (i) direct intervention strategies that 66 rely on fundamental physio-environmental changes (e.g., carrying salt masses out of the reservoir, disconnecting the reservoir water from salt masses by coating such as geo-membrane, building a 67

clay blanket to decelerate the dissolution rate, and laying transmission pipelines in the reservoir 68 69 bottom to convey salinity to the Persian Gulf), and (ii) practices of crisis management including 70 reservoir water salinity management without performing any remedial operations (IPRC, 2011; 71 MGCEC, 2012). Given the direct intervention strategies proving ineffective or having adverse 72 effects, the low-cost management and operation of the dam reservoir is a viable alternative to 73 reduce reservoir outlet salinity without further interventions in the downstream environment. These 74 management-based practices aim to resolve the existing problem with maximum final performance 75 and minimum execution and operational interference (IPRC, 2011; MGCEC, 2012; Aghasian et al., 76 2019). They typically have fewer consequences and costs, both economically and operationally 77 (Naderkhanloo, 2013; Aghasian et al., 2019).

78 This study developed a useful management-based strategy using the MIKE3 model to 79 control and resolve the salinity crisis at the Gotvand dam reservoir under current deteriorating 80 conditions. From the viewpoint of geometry, topography, and the presence of GEF, the Gotvand 81 reservoir is a complex water body with the outcropped GEF in different parts of the reservoir area 82 in triple directions of x, y, and z. Therefore, we modelled the GEF as the salinity source that affects 83 both the plan (x and y directions) and the reservoir's depth (z direction). Given that the MIKE3 84 model is not simply sophisticated to deal with nonpoint sources of pollution, such as the outcropped 85 evaporite formations, we introduced the GEF to the model as a collection of point sources. Another challenge in introducing the GEF into the mathematical model is our poor understanding of the 86 87 GEF dissolution rate (Aghasian et al., 2019) – this important parameter varies in both space and time. In general, it is not recommended to use the suggested dissolution rates for different evaporite 88 89 formations due to the high range of uncertainty reported in the related studies conducted globally (Raines and Dewers, 1997; Klimchouk and Aksem, 2002; Baghdardokht and Heidari, 2005; 90 91 Aljubouri and Al-Kawaz, 2007; Mbogoro et al., 2011; Valor et al., 2011; Lebedev, 2015; 92 Domínguez-Villar et al., 2017; Feng et al., 2017; Hong et al., 2018; Tang et al., 2018; Tavoosi et 93 al., 2022). The exact solution to understand the GEF dissolution rate in lakes/reservoirs is through 94 laboratory tests or field measurements. In the case of the Gotvand reservoir, it was difficult to 95 determine the exact dissolution rate due to the impossibility of physical models that incorporate 96 natural conditions in the process. Here, we estimated the GEF dissolution rate using salinity 97 calibration of the mathematical model in the Gotvand reservoir. Contrary to similar studies (e.g., 98 Aghasian et al., 2019; Tavoosi et al., 2022), we considered the GEF dissolution rate as a space-time 99 dependent variable in the model to appropriately highlight the impact of different impoundment 100 stages on the GEF dissolution in the reservoir.

101 Everything considered, we examined how water extraction conditions from different levels 102 of the Gotvand hypersaline dam reservoir simultaneously affect reservoir water salinity and 103 downstream of the dam. Our findings revealed that the undesirable impact of the GEF on the 104 reservoir will persist for a period in the future. However, salt mass volume will decrease due to its 105 continuous dissolution over time.

106 2 Study area

107 2.1. General description

108 Gotvand, with a height of 182 m, is the tallest Iranian embankment dam. It is located at 32° 15' 59" N and 48° 55' 51" E, northeast of Gotvand city, Khuzestan province, in southwest Iran. 109 This dam, with a storage capacity of around 4.7 km³ at the normal operating level, was built about 110 380 km away from the Karun's river mouth at the Persian Gulf in 2011 (Fig. 1a) (IPRC, 2011; 111 IWPRDC, 2011; MGCEC, 2012; Jalali et al., 2019). Karun is the largest and most important river 112 113 in Iran, with a crucial role in hydropower generation and water supply for irrigation, industry, and sanitary uses. Five dams are constructed along this river, with a total storage capacity of around 114 115 13.1 km³, making it the most regulated river in Iran. The Gotvand is the last (location-wise)

constructed dam on this river (IWPRDC, 2011; Aghasian et al., 2019; Fakouri et al., 2019). It was 116 117 constructed to increase the country's energy production capacity and promote its international position in terms of energy security, indigenization of related technologies, flood control, water 118 119 supply for different uses (especially drinking water for urban residents), and social welfare of the 120 surrounding region (IWPRDC, 2011). The highest rate of hydroelectric power generation in Iran 121 belongs to this dam (4250 GWh per year). Compared to the other related industries involved in the Gotvand project, farmers' position seems far more sensitive since a million hectares of farmlands 122 123 and thousands of farming operators exist downstream.

Detailed information about the course of development and social, political, and environmental effects connected to the Gotvand dam project is given in Text S1.

126 2.2. The reservoir salinization

During the impoundment phase of the Gotvand dam, the vast and large GEF within the reservoir (Fig. 1) began to dissolve, making it increasingly saline. The GEF extends 4.5 km upstream from the dam axis (Figs. 1b to 1d) (MGCEC, 2012). Measurements demonstrate that the degree of water salinity, in terms of electrical conductivity (EC), at 11 km from the Gotvand downstream is nearly 1200 μ mhos/cm. However, the salinity of dam outlet water is still at an optimum level compared to the desired maximum level (1650 μ mhos/cm), allowing a maximum concentration for irrigation use (2500 μ mhos/cm) (Naderkhanloo, 2013).



134

Figure 1. (a) The location of the Gotvand dam in the Karun river basin, Khuzestan province, Iran; and (b, c, and d) the location of the Gachsaran evaporite formation (GEF) on the left bank of the Gotvand dam reservoir.

138 To properly mitigate the negative impacts of the Gotvand reservoir impoundment, two 139 salient points should be addressed: (i) the degree of water salinity downstream should be 140 considered, and (ii) there is no guarantee that the outlet water maintains its optimum quality. Upon 141 reservoir impoundment, the water level at the dam's location increased from 80-90 m (river water 142 level) to 230 m above sea level (a.s.l) at the normal operating level. During this phase, the GEF, 143 which mainly consists of gypsum, marl, and salt masses, at the left abutment of the dam (Fig. 1b) 144 was partially submerged, and the dissolution process of salt was started. The GEF dissolution will 145 proceed through four subsequent main mechanisms (IPRC, 2011):

- Dissolution of the outcropped GEF (scattered throughout the lateral surface of the reservoir) by its direct contact with reservoir water and causing free saline flow conveyance on this surface (simultaneously from the outset of impoundment),
- Dissolution of the GEF existing in the walls of karst cavities and its entry into the reservoir owing to concentration gradient (after the start of impoundment),
 - Dissolution of the GEF and their entry into the reservoir simultaneously with a decline in water surface elevation arising from hydraulic gradient (peculiar to operation period),
- Dissolution of the GEF because of instability, fall, and partial slippage of masses into the reservoir (during operation and over the impoundment period, depending on the degree of salt dissolution).

In addition to the GEF, three small saline tributaries that join the Karun River at the Gotvand dam upstream (i.e., Murghab, Andika, and Lali tributaries) further contribute to reservoir salinization (Fig. S1 and Table S2). These tributaries are around 3 to 17 times more saline than the Karun river.

160 **3 Numerical simulation**

151 152

Thorough knowledge of flow hydrodynamics, salinity layering, and salt accumulation is needed to 161 exert salinity management of reservoir water and explore its feasibility. Here, we outline the 162 163 mathematical modelling stages and required data/parameters for the Gotvand reservoir salinity 164 simulation. The mathematical model was set up for the Gotvand dam reservoir to call the primary data (hydrological, climatological, topographical, and water salinity data) and the constructed 165 166 meshes. After that, the model was calibrated and verified based on water surface elevation and insitu measured depth profiles of water temperature and salinity to ensure the model's performance. 167 168 Finally, the tuned mathematical model was run for reservoir salinity stratification and accumulation 169 and evaluating different salinity management scenarios in the dam downstream.

170 *3.1. Salinity transport model and governing equations*

171 In light of their dimensions, dam reservoir hydrodynamics and salinity distribution vary in 172 lateral, vertical, and longitudinal directions. Given the complex geometry and topography of the Gotvand dam reservoir and the 3D distribution of the GEF in the reservoir area (see Figs. S2 and 173 **S3**), we employed the mathematical model of MIKE3 to further investigate the salinity crisis in this 174 175 reservoir. This model has been successfully applied for 3D simulation of hydrodynamics and water quality around the world (Bedri et al., 2014; Kheirabadi et al., 2018; Ranjbar et al., 2020 and 2022). 176 177 MIKE 3 model can consider the entire evaporite formation face, which extends at the reservoir's 178 depth as the salinity source. Additionally, the model determines the salinity concentration at 179 different widths and depths of the reservoir and its outlets, leading to reliable outputs for the salinity 180 management in Gotvand hypersaline reservoir.

181 In this study, only hydrostatic pressure was assumed, and the calculation of rotational 182 currents in plan and depth and velocity changes were performed in three dimensions. The equations 183 discussed above, thus, are presented as follows (DHI, 2017):

184 • Continuity equation

185
$$\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} + \frac{\partial w}{\partial z} = S$$
 (1)

where, x, y, and z are the Cartesian coordinates in lateral, longitudinal, and vertical directions, respectively; u, v, and w represent the flow velocity components in the directions of x, y, and z, respectively; and S is the value of point source discharges.

189 • Momentum equations

190 Momentum equations along *x* and *y* axes are described as follows:

$$\frac{\partial u}{\partial t} + \frac{\partial u^2}{\partial x} + \frac{\partial vu}{\partial y} + \frac{\partial wu}{\partial z} = fv - g\frac{\partial \eta}{\partial x} - \frac{1}{\rho_0}\frac{\partial P_a}{\partial x}$$

$$191 - \frac{g}{\rho_0}\int_z^{\eta}\frac{\partial \rho}{\partial x}dz - \frac{1}{\rho_0h}\left(\frac{\partial s_{xx}}{\partial x} + \frac{\partial s_{xy}}{\partial y}\right) + F_u + \frac{\partial}{\partial z}\left(v_t\frac{\partial u}{\partial z}\right) + u_sS$$

$$\frac{\partial v}{\partial t} + \frac{\partial v^2}{\partial y} + \frac{\partial uv}{\partial x} + \frac{\partial wv}{\partial z} = -fu - g\frac{\partial \eta}{\partial y} - \frac{1}{\rho_0}\frac{\partial P_a}{\partial y}$$

$$(2)$$

192
$$-\frac{g}{\rho_0} \int_z^{\eta} \frac{\partial \rho}{\partial y} dz - \frac{1}{\rho_0 h} \left(\frac{\partial s_{yx}}{\partial x} + \frac{\partial s_{yy}}{\partial y} \right) + F_v + \frac{\partial}{\partial z} \left(v_t \frac{\partial v}{\partial z} \right) + v_s S$$
(3)

193 where, t is the time. $h = \eta + d$, where η is the surface elevation, and d stands for the still water depth. 194 $f = 2\Omega \sin \phi$ reflects the Coriolis force, where Ω is the angular revolution rate, and ϕ denotes the geographical latitude. Gravitational acceleration and water density are shown by g and ρ , 195 respectively. s_{yy} , s_{yx} , s_{xy} , and s_{xx} are components of the radiation stress tensor. The vertical eddy 196 197 (turbulent) viscosity is shown by v_t . P_a and ρ_0 stand for the atmospheric pressure and water reference density, respectively. S is the value of point source discharge; v_s and u_s are the water 198 199 velocity induced by the point sources along x and y directions. F_{v} and F_{u} are the horizontal stress 200 terms that are expressed as:

201
$$F_{u} = \frac{\partial}{\partial x} \left(2A \frac{\partial u}{\partial x} \right) + \frac{\partial}{\partial y} \left(A \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) \right)$$
(4)

202
$$F_{\nu} = \frac{\partial}{\partial y} \left(2A \frac{\partial \nu}{\partial y} \right) + \frac{\partial}{\partial x} \left(A \left(\frac{\partial u}{\partial y} + \frac{\partial \nu}{\partial x} \right) \right)$$
(5)

203 In the above relation, *A* indicates the horizontal eddy viscosity.

• Salinity advection-dispersion equation

205
$$\frac{\partial s}{\partial t} + \frac{\partial us}{\partial x} + \frac{\partial vs}{\partial y} + \frac{\partial ws}{\partial z} = F_s - \frac{\partial}{\partial z} \left(D_v \frac{\partial s}{\partial z} \right) + \hat{H} + s_s S$$
 (6)

where, D_v is the vertical turbulent (eddy) diffusion coefficient, \tilde{H} is a source component arising from atmospheric temperature changes, s_s is point source salinity, F_s is the component of horizontal salinity diffusion that is expressed by:

209
$$F_{s} = \left[\frac{\partial}{\partial x} \left(D_{h} \frac{\partial}{\partial x}\right) + \frac{\partial}{\partial y} \left(D_{h} \frac{\partial}{\partial y}\right)\right](s)$$
(7)

210 where D_h stands for horizontal eddy diffusion.

211 *3.2. Model configuration*

The first requirement for setting up a 3D model is importing topographical data for cell mesh generation into the model. A combined mesh generation technique is applied to the problem solution domain to reduce the execution time of the model and to foster fewer instabilities (DHI, 2017). Two flexible rectangular and triangular grids are implemented to superimpose horizontal

- 216 grids onto the solution domain. The river channel is discretized with high-density rectangular grids
- 217 (50×200 m) and triangular grids, consisting of 7238 elements and 4773 nodes, spread over the
- reservoir's surface (Fig. 2). Within the reservoir, as it goes deeper, the grids get denser to account for the extreme salinity gradient at lower elevations more accurately.



220

Figure 2. Horizontal (i.e., longitudinal and lateral) and vertical mesh generation of the Gotvand dam reservoir.

For vertical mesh generation of the solution domain of 46 deep layers, a combination of 40 Z-Level and 6 Sigma layers of the same thickness was designated. In deep Sigma mesh generation, layer thickness at different points varies depending on the rise and fall of the bottom, while Z-Level mesh generation has a constant thickness (Naderkhanloo, 2013; DHI, 2017). As the main flow course is along the river, the reservoir gets wet and dry sporadically during the impoundment, and the flow constantly fluctuates.

229 3.3. Measurement data and boundary/initial conditions

For setting up the MIKE3 mathematical model, some measured hydrological, climatological, topographical, and water quality data are required (see Table 1). In our study, these data were collected, validated, and prepared to be introduced to the model, which involved considerable time, financial resources, and high-tech devices. Salinity and water temperature data were gathered periodically using an electromechanical device with various sensors. The sensors were positioned at elevations of 80, 90, 95, 100, and 120 m above sea level for almost the first year of impoundment, from 23/7/2011 to 18/6/2012.

237 **Table 1.** Primary measurement data used in the three-dimensional (3D) model (MIKE3).

Parameter/Input	Reference	
Reservoir bathymetry (topography)	30 m resolution DEM	
Air temperature		
Relative humidity	Cotword apportio station	
Rainfall and evaporation	Gotvand synoptic station	
Wind speed and direction		
Karun river discharge and salinity (upstream boundary	Gotvand hydrometric	

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condition)	station
Discharge and salinity of saline rivers entering the reservoir GEF properties	(MGCEC, 2012)
Reservoir salinity in various depths and times Reservoir water level	Direct measurement

238 Due to the substantially high storage capacity of the Gotvand dam reservoir and the 239 selection of a multi-stage impoundment process for the examination of GEF behaviour, the impoundment conditions were facilitated with the aid of various upstream dams of the Gotvand 240 241 (i.e., Karun 4, Karun 3, Abbaspour and Masjed Soleyman dams, respectively). On occasions of a multi-stage impoundment, the increase in the output of the mentioned dams boosted the process of 242 243 raising the water surface elevation of the Gotvand reservoir. Considering that such a process was specially designed for the particular case of the Gotvand dam, it is formulated based on time periods 244 and different volumetric water yields back to the dam. Nevertheless, initiations and suspensions of 245 246 impoundment are a matter of great importance. In normal circumstances and other situations, multiple starts and stops are rare during impoundment. Considering this, supervisors and operators 247 248 of the Gotvand dam monitored and controlled conditions of GEF dissolution and the reservoir water 249 quality by resorting to a kind of impoundment with periodic timing, time range, and variable 250 volume. At each stage and provided that the situation was risk-free, they planned for the subsequent 251 impoundment and its magnitude, both in time and volume. Given that the impoundment of the 252 Gotvand reservoir was started in July 2011, and the monthly average discharge of the Karun River into the Gotvand reservoir in this month was 250 m³/s (Fig. S4), this discharge value was used to 253 254 set out the initial condition of the model before the start of impoundment.

The average salinity of the Karun river at the dam location before the impoundment was considered 1000 µmhos/cm, equal to the historical salinity of the river (MGCEC, 2012; Jalali et al., 2019). Impoundment of the dam began on 28/07/2011 and progressed through four stages up to 140, 160, 185, and 205 m. The reservoir's inflow and the bottom outlet discharge were set up to match the upstream and downstream boundary conditions during the impoundment period. The tributaries' average discharge and salinity were introduced to the model as point sources of salinity (Table S2 and Fig. S1).

262 *3.4. Introduction of the GEF into the model*

263 Since the MIKE3 model only accepts the point source option, we introduced the nonpoint 264 source pollution of GEF to the model as a collection of point sources. In this regard, 945 point sources were considered over the entire evaporite formation interface (see Fig. S3), leading to the 265 spatially varying dissolution rate of the GEF. Each point source required two characteristics for 266 input into the model: the discharge and concentration; however, only the mass loading rate was 267 268 available. Therefore, we included a hypothetical discharge and then divided the mass loading rate by the hypothetical discharge to calculate the source concentration. Then, we eliminated the 269 270 hypothetical discharge effect (because no flow from evaporate formations enters the dam reservoir) 271 by considering a source with a negative discharge (actually a sink) and a zero concentration. In this 272 way, we successfully modelled the mass loading rate from the entire evaporite formation interface.

273 *3.5. Model calibration and validation*

Our in-situ measured salinity and water temperature data were collected from 28/07/2011 to 19/06/2012 for 327 consecutive days. Data from some elevations (i.e., 80 and 120 m) were used for calibration, while data from other elevations (i.e., 90, 95, 100, and 160 m) were employed for the numerical model validation. Then, the calibrated and validated model was used to simulate salinity in the Gotvand reservoir from 28/07/2011 to 02/18/2015 for 1332 days. The roughness coefficient value was verified using the measured water elevation in the reservoir, available from 28/07/2011 to 21/05/2013 for 664 consecutive days. Along the length of the reservoir (from the beginning of the solution domain to the dam axis), three different values of bottom roughness, which also represent the Manning coefficient, were applied for sensitivity analysis. In addition to the bottom roughness and Manning coefficients, other hydrodynamic parameters were selected to analyse the model sensitivity, like time step, flood and dry depth, eddy viscosity, and dispersion coefficient.

286 The GEF dissolution rate calibration plays a crucial role in reservoir salinity simulation. The 287 effect of hydrodynamics, salinity and temperature gradients, pH, hydraulic pressure, and 288 heterogeneity of the masses within the reservoir contribute to the complexity of the GEF dissolution. Hence, simple relations and dissolution rates in previous studies are case-dependent and 289 290 cannot be generalised elsewhere (Tavoosi et al., 2022). Accordingly, we calibrated the changes in 291 the GEF dissolution rate by comparing the measured data of the reservoir salinity with modelling 292 results. The measured salinity data at elevations of 80 and 120 meters were used for calibrating the 293 model, and the measured salinity data at elevations of 90, 95, 100, and 160 meters were used for 294 validation.

295 **4 Reservoir management strategies**

296 According to measurements, the salinity value entering the Gotvand dam reservoir is around 1000 297 µmhos/cm. As discussed, this salinity value represents the average historical salinity of the Karun 298 River before the dam construction. Therefore, the dam's outlet water quality control (outlet loading) 299 must be closely monitored to prevent salinity accumulation and role-playing of the GEF (in addition 300 to salinity entering the reservoir from upstream) in the Gotvand dam reservoir after impoundment. 301 In terms of implementation and operation, any salinity concentration is considered an outlet salinity 302 that can be released. However, these values should be logically balanced to mitigate and control 303 downstream damaging effects and avert incremental salinity accumulation in the dam reservoir. The release of the same salinity at 1000 µmhos/cm over time leads to excessive salinity accumulation in 304 305 the dam reservoir due to submerged GEF dissolution. If this volume of salinity increases, output water becomes saline, and sluices and outlet structures will face some problems. As a result, values 306 307 higher than the above should be considered as loading output under technical and control 308 considerations. Then, continuous monitoring can prevent any possible environmental and structural 309 problems in the future.

Overall, a safe value of salinity should be guided downstream. This value needs to account for the effects of both upstream salinity and salinity induced by the GEF submergence. Therefore, some salinity caused by the GEF should be added to upstream salinity. Then, the state of accumulated salt in the reservoir needs to be inspected. Here, three scenarios for behavioural investigation of accumulated salts in the reservoir—which itself is affected by the dam's salinity outlet—are explored, and their details are given below.

Conveying good-quality water downstream while paying particular attention to the 316 qualitative conditions of the Gotvand dam and preventing salt accumulation is a complicated series 317 of steps formulated within the framework of water salinity management in the reservoir. Even 318 though this managerial aspect does not call for structural costs and undue physical interventions, it 319 320 demands exact control plans, continuous monitoring of water salinity conditions from the reservoir 321 to downstream, and following sensitive and decisive operational points. Consequently, employing 322 the full structural capacity of the Gotvand and focusing on outlet water of varying volume and salinity can lead to the further success of this remedial strategy. Fig. 3 presents details of the type 323 324 and elevation of Gotvand outlets. The dam has three outlets at different elevations, namely: bottom 325 pipe or GRP pipe at 90 m elevation, lower outlet at 123 m elevation, and power plant inlets at 158 m elevation. It is also possible to convey water downstream via GRP pipe in water diversion 326 tunnels at 110 m elevation (MGCEC, 2012). It is possible to simultaneously improve dam reservoir 327

328 and downstream conditions by continuously utilising the limited or total capacity of these sluices,

329 whether individually or integrally. Whereas lower reservoir elevations contain high salinity, upper 330 elevations have a good water quality condition in terms of salinity—sometimes lower than the

331 permissible limit. The only viable solution until the problem is completely resolved is to develop

detailed plans for concurrently employing all these capacities.





Figure 3. Schematic diagram of the outlets and their corresponding elevations in the Gotvand dam reservoir.

Here, the total outlet salinity loading is drawn up in three different scenarios: (i) 1200 μ mhos/cm, (ii) 1300 μ mhos/cm, and (iii) 1400 μ mhos/cm, which are more than the upstream salinity (i.e., 1000 μ mhos/cm). In each scenario, a constant value of salinity loading from all the outlets as the final salinity loads to the downstream are considered, as shown in Eqs. (8) and (9).

340	UL + SFL = PIL + LOL + BOL	(8)
341	BO = DTO + GRP	(9)

where, UL is the upstream loading, SFL is the GEF loading, PI is the power plant inlet, LO is the
lower outlet, BO is the bottom outlet, DTO is the diversion tunnel outlet, and GRP is the GRP
outlet at the lowest level.

345 **5 Results and discussion**

346 5.1. Results of salinity measurements in the Gotvand reservoir

347 With the submergence and GEF dissolution in the reservoir, the bottom layers at low elevations became increasingly saline. The salinity accumulation steadily continued from the 348 reservoir bottom to the 90 m elevation within this process. After two months of impoundment, the 349 water level reached an elevation of 140 m when a strong halocline was observed in the water 350 column. Also, the measured salinity was very high at the monimolimnion (i.e., 94.1 and 42.8 ppt at 351 80 and 90 m elevations, respectively). In contrast, the upper layers were less impacted by salinity 352 353 (salinity was 9, 1.7, and 0.4 ppt at 95, 100, and 120 m elevations, respectively). The salinity remained constant at the reservoir bottom (i.e., 80 m elevation), while incremental salinization 354 355 started from 21/01/2012 and continued by the end of field measurements, i.e., 19/06/2012. Within 356 this period, salinity at 90, 95, 100, and 120 m elevations increased to 94.6, 53.4, 28.2, and 5.1 ppt, respectively (Fig. 4a), causing a crenogenic meromixis condition in the Gotvand reservoir. Under 357 this condition, the Gotvand reservoir was increasingly stratified because of the introduction of an 358

uninterrupted source of saline water to the bottom layers of the lake resulting from GEF dissolution(Hutchinson, 1937; Tavoosi et al., 2022).



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Figure 4. (a) Salinity calibration (elevations of 80 m and 120 m) and validation (elevations of 90 m, 95, and 100 m) results in the Gotvand dam reservoir from 28/07/2011 to 19/06/2012, (b) the measured and modelled water surface elevation in the Gotvand dam reservoir during the different impoundment stages, and (c) the measured and modelled salinity values at two elevations of 80 m and 120 m in the calibration stage and the corresponding calibrated dissolution rates of the Gachsaran evaporite formation (GEF).

367 *5.2. Hydrodynamics calibration and validation results*

368 Calibration results of water level elevation in the reservoir are shown in Fig. 4b, with a mean absolute error (MAE) of 3.62 m. As shown in Fig. 4b, the MIKE3 model overestimates the 369 370 water levels in the Gotvand reservoir with relatively considerable differences between the simulated and observed water levels. Sparse temporal frequency data with short duration could create many 371 challenges for hydrodynamic simulations, especially in the case of the Gotvand dam that is 372 373 confidential with few available data. However, the simulated results appropriately follow the 374 available trend in the measured water level. Also, the calibrated bottom roughness and Manning coefficients along different lengths of the reservoir are presented in Table 2. The simulation results 375 376 were not highly sensitive to other parameters due to the reservoir's relatively low flow velocity and 377 the accuracy of the sensitivity analysis. Therefore, the previously recommended values for 378 calibration parameters were used (see Table 3).

Given that water temperature is one of the influencing factors on the density value, which also affects the hydrodynamics of the reservoir, we investigated the depth profiles of simulated water temperature against those measured in the depth of the Gotvand reservoir. Our results revealed only one annual mixing in the reservoir during cold months, mainly December to March, putting the reservoir in a warm monomictic state (Noori et al., 2019). The MAE values for water temperature calibration and validation were 2.2 °C and 3.6 °C, respectively.

385 Table 2. Calibrated values of the bottom roughness and Manning coefficients during the 386 impoundment period of the Gotvand dam reservoir.

Parameter

Longitudinal distance (km)

	0-30	30-60	60-90
Manning coefficient	0.03	0.035	0.04
Bottom roughness coefficient	0.196	0.494	1.1

387 Table 3. Recommended values for the calibration parameters of the MIKE3 model used in our388 study.

Parameter name	Explanations	Value	
Time step	Based on test results of the independence of the time steps	1 (hour)	
Flood and dry	Based on calibration and the	DHI advise Calibrated	
a: Drying depthb: Wetting depthc: Flooding depth	: Drying depth DHI advise : Wetting depth : Flooding depth	a = 0.005 (m) $a = 0.05 (m)$ $b = 0.05 (m)$ $b = 0.07 (m)$ $c = 0.1 (m)$ $c = 0.1 (m)$	
Eddy viscosity	a: Based on the DHI advise	DHI advise Considered	
a: Horizontal eddy viscosity	Smagorinsky formulation)		
b: Vertical eddy viscosity	b: Vertical eddy viscosity	a = 0.1 to 0.3 $a = 0.1$	
	b: Based on the DHI advise and solution status (the log law formulation)	b = 0.1 to 0.4 b = 0.1 (m ² /sec) (m ² /sec)	
Dispersion a: Based on the DHI advis		$a = 0.01 \times Horizontal eddy$	
a: Horizontal dispersion	and solution status (a ratio of horizontal eddy viscosity)	viscosity (m^{-}/sec)	
b: Vertical dispersion	$b = 0 (m^2/sec)$		
	b: Based on the DHI advise and solution status (a ratio of vertical eddy viscosity)		

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5.3. Dissolution rate calibration results

391 Contrary to similar studies (e.g., Aghasian et al., 2019; Tavoosi et al., 2022), calibration 392 results revealed that the dissolution rate varied depending on the different impoundment periods. 393 The lowest value was 0.5 cm/h, while the highest was 7 cm/h (Fig. 4c). Overall, the most decisive 394 factor responsible for this range of dissolution rate variations in the Gotvand dam is hydrodynamic flow changes in the reservoir. Based on 3D changes induced by various stages of impoundment, the 395 396 dissolution rate is variable until it reaches 0.5 cm/hr, primarily because the water surface in the reservoir reaches the designed elevation, and the entirety of the GEF gets submerged. While we did 397 398 not investigate the influence of other factors on the GEF dissolution rate (e.g., pH, water temperature, and salt saturation concentration) (Raines and Dewers, 1997; Lasaga, 1998; Jeschke et 399 400 al., 2001; Mbogoro et al., 2011; Lebedev, 2015), we hypothesised that a change in these factors 401 during the Gotvand dam impoundment could contribute to the temporal variation of the dissolution402 rate.

403 *5.4. Salinity calibration and validation results*

404 The MAE values for salinity calibration and validation were 1.9 and 3.2 umohs/cm. 405 respectively. Given the large variation of salinity in the Gotvand dam reservoir (up to 100,000 406 umohs/cm in deep layers), the calculated MAE values are satisfactory. Also, the simulated depth 407 profiles of salinity appropriately follow the measured salinity profiles in the Gotvand reservoir (Fig. 408 4a). Accordingly, the MIKE3 model simulates salinity in the Gotvand dam reservoir more 409 accurately than water temperature. Sparse temporal frequency data with short duration could reduce 410 the salinity calibration accuracy, especially in the case of the Gotvand dam that is confidential with 411 few available data. In addition, complex nature of the reservoir and outcropped GEF, the complexity of the problem-solving environment, and the arrangement and introduction of GEF to 412 413 the 3D model could reduce the salinity calibration acuraccy in the Gotvand hypersaline reservoir.

Salinity and temperature gradients in the reservoir depth are almost inversely correlated. It is 414 415 evident that the salinity gradient at the bottom of the reservoir is very high, as demonstrated by the measurement data (Fig. 4a). Thus, we introduced thinner layers at the bottom of the reservoir to 416 improve the accuracy of salinity simulations. Due to the thermocline, the water temperature 417 418 gradient in the bottom layers (hypolimnion) is very low. Consequently, the bottom layers should be 419 coarser than those in the surface and middle layers. As a result, the number of vertical layers should 420 be increased to capture both salinity and temperature gradients accurately. This process 421 exponentially increases the model calculation time, which is considered a main obstacle in 422 hydrodynamics and salinity simulation in a large and deep reservoir such as the Gotvand dam 423 reservoir. Since salinity concentration was our target parameter, we only increased the number of 424 bottom layers. However, it should be noted that this layering arrangement allowed us to capture the 425 water temperature gradients in the depth of the reservoir adequately with an overall acceptable AME, as discussed before. 426

427 5.5. Salinity stratification and accumulation

428 The GEF gets dissolved due to increased water surface elevation from the early 429 impoundment. By generating density current and moving forward, the densely dissolved GEF 430 settled on the lower layers of the reservoir. This process led to salinity accumulation up to the 431 elevations of 80 and even 90 m for three months. As water surface elevation remained constant for 432 four months, the GEF dissolution rate declined, and the current low dissolution rate merely maintained the salinity of all elevations. Re-impoundment of the dam on 21/01/2012-in other 433 words, an increase in water surface elevation and inlet-outlet flows during February and March-434 435 heightened the GEF dissolution and led to salinity accumulation and escalation at upper reservoir elevations (Fig. 4a). It is noteworthy that, at the end of the period and near the reservoir bottom, the 436 salinity was around 100 ppt, which is virtually three times higher than that of the high-seas. In sum, 437 438 MIKE 3 simulation results show a crenogenic meromictic condition started in the reservoir from its 439 beginning impoundment which remained stable during the study period, even December to March 440 when the Gotvand reservoir is homo-thermal. In other words, the Gotvand reservoir is stratified 441 chemically due to a continuous source of dissolved GEF in the deep layers of the reservoir. The 442 salinity gradient in the water column shaped the mixolimnion and monimolimnion in the top and 443 deep layers, respectively. Both layers are disconnected by a distinct halocline that contributes to 444 different salt concentrations in the bottom and surface layers in the reservoir (Fig. 4a). Therefore, 445 deep hypersaline layers in the reservoir do not simply mix with the surface layers using external forces such as extreme floods. For example, our simulation results revealed that the halocline 446 447 prohibits the mixing between deep and surface layers, even under the condition of a 1000-year flood as inflow to the reservoir, which is consistent with the reported results for 100-year and 1000year floods in another salinized reservoir (Tavoosi et al., 2022).

450 Figure 5 shows graphs of salinity loading caused by GEF dissolution in the reservoir (from the first stage of the impoundment to the last stage) and accumulated salt in the reservoir (observed 451 452 and simulated by the 3D mathematical model). In light of the configuration and accuracy of the 3D 453 mathematical model, seamless compatibility was established between the observed values of 454 accumulated salt in the reservoir and simulated ones. Moreover, the salinity accumulation pattern in 455 the reservoir was obtained over a more extended period by extrapolating and using the results of the 456 3D simulation from the end of the simulation period (dashed line diagram). This pattern is 457 approximately a linear process increasing over time. Based on the observed values of accumulated 458 salt in the reservoir, the linear process demonstrated good consistency, reflecting the generalisation 459 accuracy of the simulated trend.

With the complete submergence of the GEF and from the end of the last impoundment 460 461 period to 25/02/2015, a constant loading induced by the total impact of these masses was observed 462 (thin red diagram in Fig. 5). The underlying cause of the constant loading is dissolution rate stabilisation, which is caused by two main factors: the complete submergence of the GEF due to 463 large-volume impoundment and reduction in salinity difference between adjacent layers (Tavoosi et 464 al., 2022). Qualitative observations and measurements in the reservoir confirm the authenticity of 465 dissolution rate stabilisation. A look at Fig. 5 also reveals the occurrence of multiple peaks in the 466 diagram of salt masses loading. These simulated peaks correspond to the different impoundment 467 stages of the reservoir (four stages). The more the volume and time length of impoundment, the 468 bigger the peaks of salt masses loading. On the other side, during periods when the impoundment's 469 volume and time length decreased, the loading diagram dropped further. As discussed earlier, the 470 reason behind the multi-stage impoundment with this frequency and volume is to study the 471 472 behaviour of GEF in contact with the dam reservoir.



Figure 5. The process and values of salt accumulation (observed and modelled) and the diagram of
salt masses loading for 43 months in the Gotvand dam reservoir.

476 5.6. Evaluation of reservoir management strategies

473

477 Here, a summary of scenarios is presented to better understand the salinity evolution in the 478 Gotvand reservoir. In each scenario, a constant value of the loading outlet, which is the final salinity outlet targeted for downstream delivery, is considered. This constant value is the sum of inlet 479 480 upstream salinity and salinity caused by GEF submergence. There are three different scenarios for 481 the loading of outlet salinity: 1200, 1300, and 1400 µmhos/cm. In this respect, the outlet salinity 482 value in all scenarios was determined based on expert observations and frequent problem-solving 483 meetings. More importantly, maximum historical downstream salinity was considered to protect 484 downstream targets from salinity hazards. In addition, tolerance of downstream targets to the 485 determined salinity—relative to different uses, mainly agricultural—is another reason for assigning 486 the mentioned value. Nevertheless, the mentioned salinity outlet should be carried out using 487 integrated water extraction from various layers to ensure that the amount of inlet/outlet salts is 488 balanced.

489 In the first scenario, the outlet salinity value (outlet loading) is 1200 µmhos/cm, slightly 490 more than that of the inlet from upstream, i.e., 1000 µmhos/cm. Figure 6a shows the total loading in 491 the Gotvand reservoir (total upstream and salt masses loadings) by a thin red line. Furthermore, 492 inlet accumulative salinity mass values in the reservoir (thick red line) and outlet accumulative 493 salinity mass values heading downstream (thick blue line) are demonstrated. On the other hand, the 494 value of outlet salinity loading from the reservoir had been 1000 µmho/cm since the beginning of 495 impoundment to a specific period. By the end of the last impoundment stage, it added up to 1200 496 µmho/cm, as shown by a dashed blue line diagram. This diagram analyses the effect of applying an 497 outlet loading of 1200 µmho/cm. Minor differences in the slopes of accumulative diagrams indicate 498 no salinity accumulation in the reservoir.

Similar explanations are valid for the second and third scenarios. The difference is that the values of outlet loading from the sluices installed in the dam body to downstream for the second and third scenarios are correspondingly 1300 and 1400 µmhos/cm. Figures 6b and 6c, illustrate the results of accumulated salt under these scenarios. Evidently, if the value of outlet loading increases, the slope of the downstream salinity accumulation diagram also increases.



Jui

⁽a)



Figure 6. Loading and accumulative values of inlet/outlet salt mass in the Gotvand Dam reservoir
under different scenarios of outlet salinity management: (a) 1200 μmhos/cm, (b) 1300 μmhos/cm,
(c) 1400 μmhos/cm.

507 The above figures vividly illustrate that the difference in inlet/outlet accumulative salt mass values remains in the reservoir as final accumulated salt. Concerning the limitation in increasing the 508 downstream outlet loading, the ultimate objective of presenting various scenarios is to lower the 509 510 final value of accumulated salt in the reservoir over time. Under these circumstances, having 511 precise information on salt mass allows us to estimate the time of the complete dissolution of the GEF in the dam reservoir and the time of the return of the dam to normality. Furthermore, obtaining 512 513 accurate information about the outlet loading value can help resolve the problem of salt 514 accumulation in the reservoir appropriately and logically. Overall, these estimations are a 515 managerial instruction to monitor the reservoir salinity continuously and meticulously until the problem is resolved. In this regard, Fig. 7 displays the effect of applying all the scenarios and prospects. As illustrated in the figure, an appropriate increase in outlet loading reduces the final accumulated salt mass in the reservoir. The effect of each scenario on the final accumulated salt in the dam reservoir is shown in three different diagrams. Here, the effect of applying each scenario on the diagram of accumulated salt is shown by time-weighted lines with negative slopes. When the relevant scenario is applied, the descending trend of each line arrives at the lowest value of accumulated salt.



523

Figure 7. The temporal process of salt accumulation inside the Gotvand dam reservoir in the first,
 second, and third scenarios.

526 Considering the limited volume of evaporite formations in the Gotvand reservoir area, the proposed scenarios aim to ensure that the salt inlet and outlet balance reduces the accumulated salt 527 528 in the reservoir (Fig. 7). Consequently, the accumulated salt in the reservoir will gradually be removed over the coming years (2040s), and the reservoir will have the same conditions as other 529 530 normal dam reservoirs. It should be noted that the mentioned years may be affected because of uncertainty related to the real volume of GEF (about 300 million m³). As a useful suggestion, it is 531 beneficial to monitor the accumulated salt in the reservoir throughout the dam's lifespan due to its 532 533 specific conditions. This will ensure that the accumulation rate is negative. In other words, the 534 reservoir gets closer to the conditions of a normal reservoir.

535 5.7. Uncertainties

Our proposed scenarios for the salinity management in the Gotvand dam reservoir depends on 536 537 various factors, including the nature of the reservoir and the Karun river system (including all upstream and downstream components), the tolerance and response of all downstream components 538 539 to the proposed scenarios, the ability of available tools and facilities, as well as the technical, 540 hardware, and operator status of the Gotvand dam. The way to propose these scenarios is actually based on the findings of this study and field facts (especially in the initial years of the dam 541 impoundment), available measured data, and advanced mathematical modeling. For example, the 542 543 findings of this study show that removing salt from the reservoir under proposed scenarios will 544 eventually improve the condition of the reservoir over the coming years (2040s) compared to the first years following the impoundment. In this case, the accumulated salt at the bottom of the 545 546 reservoir will be removed and the dam's salinity crisis will rectify. However, the important thing is, 547 something else is going on in reality. So far the salinity management of the reservoir has been based

on operational experiences. According to unpublished reports, the salinity management scenarios 548 549 have varied between EC =250 and 1950 µmhos/cm from 2011 to 2021. This means that there is a significant difference between the salinity management scenarios proposed by our research and 550 what is being currently applied. This difference causes ambiguity in imagining the future salinity 551 crisis in the reservoir by applying the implemented salinity management scenarios. In addition, lack 552 553 of adequate access to comprehensive and up-to-date salinity data in the reservoir limits the capacity 554 and scope of precise scientific studies about the future conditions of the Gotvand dam reservoir, and 555 our research is no exception. As discussed in detail, what has made this research important is 556 providing an optimal solution to solve the salinity crisis in the Gotvand dam reservoir with regard to 557 minimal access to measured data and relying on advanced mathematical modeling. Finally, as to whether the proposed scenarios and solutions can be applied in reality is a decision that needs to be 558 559 followed up and adopted by officials and the management team of the Gotvand dam

560 6. Conclusion

561 Outcropped salty geological formations within the reservoir's area significantly affect reservoir 562 water quality, dam structures, and downstream ecosystems, which may sometimes inhibit dams 563 from fulfilling their functions. Therefore, reservoir salinity management is required to mitigate the negative impacts of water quality and possible consequences for downstream environments. The 564 565 present study proposed a salinity management strategy for the Gotvand dam, where the 566 submergence of large salt masses due to the reservoir impoundment made the reservoir water increasingly saline. In this regard, time-weighted measures were taken to provide an exact 567 568 calculation of salinity stratification in the reservoir using a 3D numerical model. Various scenarios were implemented to determine the most appropriate value of downstream outlet loading to reduce 569 570 salinity accumulation in the reservoir and protect the downstream environment from salinity 571 hazards. Simulation results indicate that the reservoir salinity management technique may 572 effectively control the existing salinity crisis. Our suggested method can be viewed as a viable 573 approach by relevant managers and officials for salinity management in the Gotvand reservoir, the 574 world's most saline dam reservoir.

575 **Competing interests**

576 The authors declare no competing interests.

577 Author Contributions

578 Conceptualization: Siamak Amiri, Mehdi Mazaheri, Vahid Naderkhanloo, Jamal Mohammad Vali 579 Samani. Methodology: Siamak Amiri, Mehdi Mazaheri, Vahid Naderkhanloo, Jamal Mohammad 580 Vali Samani, Roohollah Noori. Data curation: Siamak Amiri, Mehdi Mazaheri, Vahid 581 Naderkhanloo, Jamal Mohammad Vali Samani. Formal analysis: Siamak Amiri, Mehdi Mazaheri, 582 Vahid Naderkhanloo. Investigation: Siamak Amiri, Mehdi Mazaheri, Vahid Naderkhanloo, Jamal Mohammad Vali Samani, Roohollah Noori. Resources: Mehdi Mazaheri, Jamal Mohammad Vali 583 Samani. Software: Siamak Amiri, Mehdi Mazaheri, Vahid Naderkhanloo. Supervision: Mehdi 584 585 Mazaheri, Jamal Mohammad Vali Samani, Ali Torabi Haghighi, Roohollah Noori. Validation: 586 Siamak Amiri, Mehdi Mazaheri, Vahid Naderkhanloo, Roohollah Noori. Visualization: Siamak 587 Amiri, Mehdi Mazaheri, Vahid Naderkhanloo, Jamal Mohammad Vali Samani, Roohollah Noori. 588 Writing - original draft: Siamak Amiri, Mehdi Mazaheri, Vahid Naderkhanloo. Writing - review 589 & editing: Siamak Amiri, Mehdi Mazaheri, Vahid Naderkhanloo, Jamal Mohammad Vali Samani, 590 Sahand Ghadimi, Ali Torabi Haghighi, Roohollah Noori.

591 Data Availability Statement

592 Data used in our study are included in a "Zenodo" repository, available through 593 <u>https://doi.org/10.5281/zenodo.8403987</u>.

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 <u>https://doi.org/10.3390/w13030296</u>

SUPPLEMENTARY MATERIALS FOR:

Salinity Management in the World's Most Saline Dam Reservoir: The Gotvand Reservoir, Iran

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Contents of this file

Text S1 Tables S1 and S2 Figure S1 – S4 References

Text S1

The course of developments and social, political, and environmental effects connected to the project of the Gotvand Dam: The Gotvand is the tallest embankment dam in Iran, which is built at a distance of 380 km from the river mouth of Karun (the most important river in Iran). Located 10 km northeast of Gotvand city in Khuzestan, a province in southwest Iran, the Gotvand is the last dam constructed on this river. The Khuzestan Plain and important cities such as Ahwaz, Abadan, Shooshtar, Khorramshahr, and Gotvand are situated downstream of the dam. The Gotvand Dam is Iran's second-largest and largest water reservoir on the Karun River. Furthermore, the highest rate of hydroelectric power generation among the country's hydroelectric power plants belongs to this dam (4250 GWh per year). The primary aims of the construction of the dam include an increase in the country's capacity for electricity generation, growth and promotion of the country's international position in terms of developments in electricity and energy fields, indigenization of related technologies, flood control, water supply for different uses, and job creation (IWPRDC, 2011).

By reviewing the history of the dam development, initial studies suggest that the Gotvand Dam was formerly built 15 km above the current location, which was the suitable leading site designated by foreign researchers. However, the Ministry of Energy ordered the dam site to be relocated. Apparently, the underlying reason for the dam relocation was less reservoir storage capacity, with a storage volume of over 2.2 billion m³. The final location (the current site) for the dam's construction, nevertheless, has the capacity to store 4.7 billion m^{3 of} water (IPRC, 2011).

Many economic and social experts in Iran maintain that implementing the Gotvand project (at the current location), despite technical advisories and disagreements, is due to development policies associated with reparation for Iran-Iraq war destructions. Another development policy consisted of developing hydroelectric plants to satisfy increasing domestic demands and, where possible, electricity exports to neighboring countries. Thus, officials of different governments planned and focused on creating high-walled reservoirs and generating maximum electric power using their capacity.

Unfortunately, the greatest mistake in the history of Iranian engineering was made because of the relocation of the dam construction site with the mentioned intentions. The location transfer placed a vast and large salty unit of the Gachsaran evaporite formation (GEF) within the reservoir that would inevitably submerge and salinate the reservoir water after the dam impoundment. Ultimately, the Gotvand Dam was built at the insistence of the relevant officials, and the impoundment was completed.

Many specialists involved in this project noticed the adverse effect of salt masses on the reservoir from the very beginning and communicated it to the proper authorities. Nonetheless, the cautionary observations were dismissed, and a host of documented advisories were ignored at the time, given the state of project development (IPRC, 2011). Later on, as the gravity of warnings increased, and some managerial changes were made, the issue of Gotvand water salinization was raised, and responsible organizations were ordered to investigate it. Regarding the circumstances, a technical panel of pundits and experts in different fields, including the environment, was organized. It was proved that the dam had some problems at the time, and its construction should be discontinued since the complete impoundment of the dam might pose irreversible hazards. However, the body of the dam had approximately achieved physical progress of 70% by that date, and a generous budget had been spent on its development. A similar scenario in the past was repeated; that is, scant regard was paid to warnings concerning the adverse effects of salt masses

at the Gotvand Dam, despite the possibility that they could pose an environmental threat to the area of the project's construction. The building of the dam was eventually completed.

After the completion of the construction, it did not take long for the steady unfolding of an environmental crisis in the dam reservoir to upset the expected equations in plain sight. Here, a large salty domain, namely the Gachsaran evaporite formation (GEF), dissolved in the dam reservoir and slowly made it increasingly saline. As the first remedial measure against salt masses at the dam reservoir, experts developed a large clay blanket to prevent salt penetration. With an increase in water surface elevation, however, the clay blanket collapsed at several points and had almost no impact on salinization control. The salinity of the reservoir incrementally increased in the manner that the salinity value was measured three times higher than the salinity of the high seas (Aghasian et al., 2019)

After various vicissitudes during the dam construction process and mounting disapproval by specialized institutions and experts who had warned about the issue, authorities, and organizations issued official statements regarding the project. While most administrators and involved parties supported the practical aspects and objectives of project engineering, alternative strategies were proposed to overcome the ongoing challenge. This could also be interpreted as a response to critical controversies. The authorities, for instance, withstood criticisms by introducing dam functions like water and energy supply and flood control. Most responses to a barrage of criticism by authorities of the water industry and the decision-makers of the Gotvand Dam closely reflected the reality and, in many cases, revealed determination and positive approaches. On the other side, they failed to take responsibility for a likely environmental failure. Many supporters of the dam implementation believed that although the triggered crisis of the dam might be deemed an environmental mistake, engineering mistakes seem virtually unavoidable. Hence, the calculations could differ from the expected results under some conditions. The experts who opposed the resumption and execution of the project, on the other hand, contended that insistence on launching the project under present circumstances would be the outcome of managerial mistakes made during the research phase.

As time went on, due to the importance of the challenge raised for the Gotvand Dam and its practical significance, the challenge gained national stature. Until then, dozens of sectional and cross-sectional institutions had articulated their views on the issue. The sensitivity of the Gotvand situation reached the point where the Iranian Department of Environment and General Inspection Office addressed the issue, and finally, follow-up evaluations and investigations were ordered by the first vice president (IPRC, 2011). It is worth noting that the final cost of the dam design and construction was estimated at over 3.86 billion dollars (IPRC, 2011). Regardless, dam construction advocates and optimistic predictions advocated allocating this colossal budget because it could provide tremendous economic potential. Based on expert evaluations, the benefit-cost of the plan execution was approximately equal to 2.2, which justified the acceleration in its construction by some means (IPRC, 2011). From another point of view, there was a combination of complicating risk factors that experts took into account, namely: the hefty investment fund for the project, the probability of the occurrence of unintended and long-term environmental effects, the likelihood of a decline in drinkable water quality in important cities, downstream farms, and groundwater, and also possible socio-economic consequences of evaporite formation and dissolution, salinization of reservoir water and accumulation of salt inside the reservoir. Against this background, experts were bound to put forward their ideas.

Affected by the existing problem, it seems possible that some of the stated development objectives of the Gotvand project will face administrative and operational ambiguities in the

present or the future. Management principles must be observed with absolute discretion when dealing with such a unique and unwanted situation; otherwise, the permanent conveyance of reservoir water to downstream targets may gradually reduce the quality and fertility of farms, followed by major, sometimes irreparable, repercussions.

As a million hectares of agricultural land and thousands of farming operators exist downstream, the dam water quality will significantly impact farmers' activities. Measurements demonstrate that the degree of water salinity (EC) at a distance of 11 km from the Gotvand downstream is nearly 1200 μ mhos/cm. Compared to the desirable maximum (1650 μ mhos/cm) and allowed maximum (2500 μ mhos/cm), outlet water salinity is still at an optimum level. Meanwhile, two salient points should be addressed. First, the degree of EC of pollutant sources in the water path should be considered downstream. Second, there is no guarantee that the quality of the outlet water will not degrade over time.

In this context, the position of farmers seems far more sensitive when compared with that of the other related industries involved in the Gotvand project. If adequate and necessary measures are not considered to control the reservoir water continuously, the water quality of the dam will probably suffer and gradually reduce agricultural productivity and prosperity in the region. It should be stressed that the Khuzestan Plain is one of the most important agricultural hubs in Iran. B. By way of illustration, the Aghili district, with an area of four thousand hectares of agricultural land and thousands of farming operators, is the impoundment location of the Karun River basin and the central region of the Gotvand Dam situated downstream.

Because the recommended ideas are diverse, and appropriate study conditions should be provided for each, there is no immediate and consistently effective action plan that can serve as a complete and obvious solution to the current problem. However, it should be noted that owing to the particular importance of the Gotvand Dam, in-depth studies should be undertaken by experts to identify a comprehensive and reliable method. Considering the acute conditions of the Gotvand challenge, experts in various fields regard it as a historic environmental milestone in Iranian history.

Classification of the solution	The title of the solution		
	Control of saline branches		
Solinity control from the	Control of entering salinity from Gachsaran evaporite formation (GEF)	Use of ion shield	
origin		Covering with materials	
		(submerged)	
		Electro osmosis	
Desalination and salt		Using nano	
confinement	Salt purification	ECR	
		Salt eating microorganisms	
	Salt trapping	Use of hydrogels	
		Using geobags	
	Periodic release of very saline water (quick wash)		
Reservoir management and	Gradual release of saline water from the reservoir while		
Quick wash	maintaining the salinity at the acceptable threshold (for the		
	downstream)		
	Injection to oil wells		
Transfer		Petrochemical	
Transfer	Delivery to applicant	Karun Cement,	
	industries	Ramhormoz Sodium	
	Carbonate Industries		
	Transfer to evaporation ponds near the dam		
	Evacuation to the Persian Gulf		
Water diversion and electric	Diverting water from before Gachsaran evaporite formation		
the dam	(GEF)		
une dann	Discharge through diversion dams and diversion tunnels at		
	different levels		

Table S1. Description of other remedial solutions to control the salinity of the Gotvand Dam reservoir.

Table S2. Average values of discharge and electrical conductivity (EC) of the saline tributaries (i.e., Murghab, Andika, and Lali tributaries), which are joining to the Karun river at the Gotvand reservoir upstream (MGCEC, 2012).

Tributory	Average discharge	Average EC
Indutary	(m^{3}/s)	(µmhos/cm)
Murghab	11.9	3000
Andika	4.2	15700
Lali	4.5	6000



Figure S1. The location of the saline tributaries of Murghab, Andika, and Lali, which are joining to the Karun River at the Gotvand reservoir upstream.



Figure S2. The location of Gachsaran evaporite formation (GEF) in the Gotvand Dam reservoir.



Figure S3. The location of salinity sources of evaporite formations in the 3D model.



Figure S4. The average inlet/outlet discharge values of the Gotvand dam reservoir in different months of the year.

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