Relating Hydraulic-Electrical-Elastic Properties of Natural Rock Fractures at Elevated Stress and Associated Transient Changes of Fracture Flow

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Abstract

Monitoring the hydraulic properties within subsurface fractures is vitally important in the contexts of geoengineering developments and earthquakes. Geophysical observations are promising tools for remote determination of subsurface hydraulic properties; however, quantitative interpretations are hampered by the paucity of relevant geophysical data for fractured rock masses. This study explored simultaneous changes in hydraulic and geophysical properties of natural rock fractures with increasing normal stress and correlated these property changes through coupling experiments and digital fracture simulations. We show that electrical resistivity is linked with permeability and flow area regardless of fracture roughness, whereas elastic wave velocity is roughness dependent. We also are able to categorize fracture flow patterns as aperture-dependent, aperture-independent, or disconnected flows, with transitions at specific stress levels. Elastic wave velocity offers potential for detecting the transition between aperture-dependent flow and aperture-independent flow, and resistivity is sensitive to detect the connection/disconnection of the fracture flow.

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19 Highlights:

- Changes in permeability and resistivity with stress depend on pore connectivity and are
 less sensitive to the fracture roughness.
- Changes in velocity with stress depend on the spatial distribution of asperity contacts and roughness dependency of porosity.
- Resistivity can be linked with permeability and flow area regardless of fracture roughness, whereas velocity has roughness dependence.
- Transitions of fracture flow patterns with stress define three stages that may be evident in remotely measured geophysical properties.

29 Abstract

30 Monitoring the hydraulic properties within subsurface fractures is vitally important in the contexts of geoengineering developments and seismicity. Geophysical observations are promising tools for remote 31 32 determination of subsurface hydraulic properties; however, quantitative interpretations are hampered by the 33 paucity of relevant geophysical data for fractured rock masses. This study explores simultaneous changes 34 in hydraulic and geophysical properties of natural rock fractures with increasing normal stress and 35 correlates these property changes through coupling experiments and digital fracture simulations. Our lattice 36 Boltzmann simulation reveals transitions in three-dimensional flow paths, and finite-element modeling 37 enables us to investigate the corresponding evolution of geophysical properties. We show that electrical 38 resistivity is linked with permeability and flow area regardless of fracture roughness, whereas elastic wave velocity is roughness dependent. This discrepancy arises from the different sensitivities of these quantities 39 40 to microstructure: velocity is sensitive to the spatial distribution of asperity contacts, whereas permeability 41 and resistivity are insensitive to contact distribution but instead are controlled by fluid connectivity. We also are able to categorize fracture flow patterns as aperture-dependent, aperture-independent, or 42 disconnected flows, with transitions at specific stress levels. Elastic wave velocity offers potential for 43 44 detecting the transition between aperture-dependent flow and aperture-independent flow, and resistivity is 45 sensitive to the state of connection of the fracture flow. The hydraulic-electrical-elastic relationships reported here may be beneficial for improving geophysical interpretations and may find applications in 46 47 studies of seismogenic zones and geothermal reservoirs.

Keywords: fracture flow, permeability, elastic wave velocity, resistivity, lattice Boltzmann method, digital
rock physics

50 1 Introduction

The hydraulic properties of fractured geological formations have been of interest for many purposes 51 52 such as developing fluid resources (e.g., geothermal fluids, shale oil and groundwater), geological storage or disposal, and seismic events (fault reactivation and induced seismicity). It is known that fracture 53 54 permeability and preferential flow paths within fractures are controlled by the heterogeneous distribution of apertures, which can vary as stress changes (Krantz et al. 1979; Raven and Gale 1985; Thompson and 55 Brown 1991; Watanabe et al. 2008; Ishibashi et al. 2015; Chen et al. 2017; Vogler et al. 2018). In-situ stress 56 is never constant during geoengineering developments or on the geological time scale, and consequently 57 58 the aperture distribution and associated hydraulic properties also must change in natural settings (e.g., 59 Manga et al. 2012). These changes produce transitions in the patterns of fracture flow that in turn control 60 the fault reactivation cycle (Sibson et al. 1988) and characterize the transport behavior of fluid resources.

61 Geophysical observations can detect changes in electrical resistivity or elastic wave velocity that 62 may reflect subsurface stress changes associated with hydraulic stimulation, earthquakes or geothermal fluid production (Peacock et al. 2012, 2013; Didana et al. 2017; Mazzella and Morrison 1974; Park 1991; 63 64 Gunasekera et al. 2003; Brenguier et al. 2008; Nimiya et al. 2017; Taira et al. 2018). It would be beneficial 65 if changes in aperture-related hydraulic properties triggered by subsurface stress changes could be linked to geophysical properties that can be remotely monitored. Studies based on synthetic or simulated single 66 fractures have related hydraulic properties to electrical properties (Stesky 1986; Brown 1989; Volik et al. 67 1997; Kirkby et al. 2016) and to elastic properties (Pyrak-Nolte and Morris 2000; Petrovitch et al. 2013, 68 2014; Pyrak-Nolte and Nolte 2016; Wang and Cardenas 2016). These studies have confirmed that the 69 70 relationships of these properties depend on features of the fracture microstructure (e.g., pore connectivity, 71 tortuosity, apertures and contacts), which varies with the initial fracture roughness and changes with normal 72 stress. On the one hand, connected apertures are characterized by pore connectivity and tortuosity, both of 73 which are strongly related to the permeability-resistivity relationship. On the other hand, discrete points of 74 contact (asperities) contribute to hydro-mechanical properties. Therefore, both hydraulic-electrical and 75 hydraulic-elastic relations may reflect similar microstructures; however, the underlying mechanisms do not 76 necessarily have a mutual correlation. Simultaneous measurements in identical samples may shed light on 77 the nature of variations in rock properties and their relationships. To our knowledge, no study has 78 simultaneously investigated hydraulic, electrical and elastic properties of natural rock fractures.

79 This study took advantage of recent advances in Digital Rock Physics (e.g., Tsuji et al. 2019; Sain et al. 2014) that enabled us to simultaneously determine multiple properties in the same sample while 80 81 visualizing its microstructure. In this study, we explored the simultaneous changes in fracture permeability, 82 electrical resistivity and elastic wave velocity of natural rock fractures that occur with increasing normal 83 stress. By coupling experiments and digital fracture simulations, we investigated the correlations between 84 hydraulic-electrical-elastic properties and addressed their governing mechanisms. Many studies have 85 reported a correlation between permeability and fracture specific stiffness, which is related to the amplitude 86 of the seismic response (i.e., attenuation), but have not established a direct correlation between permeability and seismic velocity (Pyrak-Nolte and Nolte 2016; Wang and Cardenas 2016). Some experimental studies 87 88 have observed velocity changes with aperture closure (e.g., Nara et al. 2011; Choi et al. 2013), but none 89 has established a direct relationship between seismic velocity and fracture permeability. As an alternative 90 to fracture specific stiffness, in this study we adopted finite-element modeling of static elasticity to calculate 91 elastic wave velocity. In this paper, we also evaluate the local behavior of the fluid flow (i.e., preferential flow paths) within fractures to investigate the connectivity of flow paths, flow area and their transient 92 93 changes. Our lattice Boltzmann simulation of digitized rock fractures reveals transitions in 3D fracture flow 94 patterns that accompany stress changes, which are difficult to observe in laboratory experiments or in the

95 field. We discuss how transient changes of the fracture flow pattern are correlated with hydraulic and 96 geophysical properties and suggest possible applications of our findings to seismogenic zones and 97 geothermal reservoirs.

98 2 Methods

99 **2.1 Sample and Experimental Procedure**

100 We evaluated the dependency of the fracture permeability on the effective normal stress in fluid 101 flow experiments. These employed two cylindrical fractured samples of Inada granite 50 mm in diameter 102 and 80 mm long, in which the fracture plane was parallel to the central axis. The two samples differed in the roughness characteristics of their fracture surfaces, as determined from the surface topographies of the 103 104 hanging wall and footwall which we mapped in a grid of cells 23.433 µm square with a 3D measuring microscope (Keyence, VR-3050). The surface of one sample, called the smooth fracture hereafter, had a 105 106 fractal dimension of 2.5 and a root mean square (rms) roughness of 1.3 mm, whereas the surface of the 107 other sample, called the rough fracture hereafter, had a fractal dimension of 2.4 and rms roughness of 1.7 108 mm (Power et al. 1987; Power and Durham 1997). The fractal dimension describes the scaling characteristics of surface topographies and is a measure of fracture surface roughness (Brown, 1995). The 109 rms roughness, called roughness hereafter, represents an rms height of fracture surface topographies. The 110 initial aperture distribution of each fracture and the corresponding probability histogram are shown in Fig. 111 112 1. Note that initial aperture models are created by numerically mating the mapped fracture surfaces, where 113 they are assumed to be in contact at one point. The aperture distribution in the smooth fracture (Fig. 1a) shows less spatial variation than that of the rough fracture (Fig. 1b). Consequently, the aperture distribution 114 115 of the smooth fracture shows a sharp peak of probability whereas the rough fracture shows a broad 116 distribution (Fig. 1c).

After measuring the fracture surfaces, we conducted fluid flow experiments on these two samples. Distilled water was injected into jacketed samples under various effective normal stresses between 5 and 30 MPa. The pore-inlet, pore-outlet, and confining-oil pressures were independently controlled by syringe pumps. For each stress state, we measured flow rates and thereby evaluated the fracture permeability based on the cubic law (e.g., Witherspoon et al. 1980), where we assumed Darcy flow and a negligible matrix permeability of granite (between 10^{-19} and 10^{-22} m²).

123 **2.2 Numerical Simulation**

124 We performed a series of numerical simulations on digitized fractures. Three-dimensional digital fracture models were prepared for each sample directly from the mapped surface topographies described 125 above in a system of 0.1 mm cubic voxels. The use of a three-dimensional fractured sample enabled us to 126 127 model local transport properties along with the rough-walled fractures. Although the voxel size potentially 128 affects absolute values of permeability and resistivity to some degree, we confirmed that a 0.1 mm voxel 129 system is small enough for our qualitative interpretations (Appendix 1). The distance between the two surfaces was adjusted in each model by uniformly reducing the local apertures so that the digitized fracture 130 had a simulated permeability equivalent to that measured in the real fractures (Watanabe et al. 2008; 131 132 Ishibashi et al. 2015).

133 Subsequently, we simulated 3D local flows within the fractures by the lattice Boltzmann method, which is suitable for modeling heterogeneous local flows with complex boundaries (He and Luo 1997; 134 135 Jiang et al. 2014). The governing equation for the lattice Boltzmann method in the D3Q19 model is given 136 by (Ahrenholz et al. 2008)

$$f_i(\boldsymbol{x} + e_i \,\Delta t, \, t + \Delta t) = f_i(\boldsymbol{x}, \, t) + \boldsymbol{\Omega}_i, \quad i = 0, \, \cdots \, 18, \tag{1}$$

where Δt is the time step and $f_i(x, t)$ is the particle distribution function that represents the probability of 138 finding a particle at node x and time t with velocity e_i . Collision operators Ω are defined by 139

140
$$\mathbf{\Omega} = \mathbf{M}^{-1} \mathbf{S}[(\mathbf{M}f) - \mathbf{m}^{eq}], \qquad (2)$$

141 where M is a transformation matrix that transforms the particle distributions into moment space. The equilibrium vector m^{eq} is composed of equilibrium moments, and the matrix S is a diagonal collision 142 143 matrix indicating the relaxation rates (Jiang et al. 2014). We implemented this model using advanced memory-saving schemes and parallel-GPU techniques to simulate digital fracture systems with a large 144 145 domain and high resolution (Jiang et al. 2014). At the fracture surfaces, bounce-back boundaries (a no-slip scheme at fluid-solid interfaces) were implemented. Provision of a constant body force from the inlet to the 146 147 outlet boundaries and the periodic boundary along the fracture plane enabled us to simulate the fracture flow (Fig. 2). Permeability along the fracture was estimated from the macroscopic flow velocity that was 148 calculated from the particle distribution function (f_i) . A series of lattice Boltzmann simulations enabled us 149 150 to explore the changes with stress state in permeability and in the flow area, defined as the ratio of the area 151 of preferential flow paths to the area of the fracture plane (Watanabe et al. 2009).

152 Once the lattice Boltzmann simulations yielded estimates of the heterogeneous distribution of flow 153 within the fracture, we evaluated both the resistivity and the elastic wave velocity by using the finiteelement method, which is a well-established method of computing rock properties from three-dimensional 154 microstructure (Garboczi 1998; Andrä et al. 2013; Saxena and Mavko 2016). Both analyses implemented 155 a periodic boundary along the fracture plane. Resistivity in the direction parallel to the fracture plane (and 156 the fluid-flow direction) was calculated from Ohm's law, where the electric current was simulated from the 157 potential difference between the inlet and outlet boundaries. Parameters used in our finite-element modeling 158 159 are summarized in Table 1. For the electrical conductivity of the solid, we used the experimental value of Inada granite under dry conditions, measured by the four-electrode method with an impedance analyzer 160 (Solartron Analytical, SI 1260A) at 10 mHz. 161

Elastic wave velocity in the direction perpendicular to the fracture plane was estimated from the simulated static elasticity under the triaxial stress state. The finite-element analysis of static elasticity enabled us to simulate the elastic wave velocity under the low-frequency limit, where a wavelength much longer than the fracture aperture was assumed. The linear stress-strain relationship is expressed as Hooke's law:

$$\sigma_i = C_{ij} \varepsilon_j, \qquad i, j = 1, \cdots 6, \tag{3}$$

where C_{ij} is the stiffness tensor (in Voigt notation). σ_i and ε_j are stress and strain tensors, both of which are solved in a finite-element analysis associated with engineered strain (Garboczi 1998). Because our fracture models can be assumed to be transversely isotropic material along the *z*-axis (perpendicular to the fracture plane), C_{ij} has five independent elements (Mavko et al. 2009):

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$$C_{ij} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & 0 & 0 & 0 \\ C_{12} & C_{11} & C_{13} & 0 & 0 & 0 \\ C_{13} & C_{13} & C_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & C_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & C_{44} & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{C_{11} - C_{12}}{2} \end{bmatrix}.$$
(4)

By solving macroscopic stress and strain in the finite-element analysis, we can estimate all of the elements of macroscopic stiffness based on Eq. (3). Thus, P-wave velocity V_p and S-wave velocity V_s in the direction perpendicular to the fracture plane are obtained from

176
$$V_p = \sqrt{\frac{C_{33}}{d}}, \quad V_s = \sqrt{\frac{C_{44}}{d}},$$
 (5)

177 where d is the average density of the solid and the fluid (Table 1). The elastic constants of the solid were

taken from experimental values; in dry, intact Inada granite under 200 MPa of confining pressure, measured

179 P- and S-wave velocities were 6.14 and 3.42 km/s, respectively.

180 To explore how geophysical properties vary with variations in the fluid distribution within 181 fractures, we investigated the correlations between fracture permeability, flow area, resistivity and elastic 182 wave velocity in detail.

183 **3 Results**

184 3.1 Changes in Fracture Permeability and Preferential Flow with Aperture Closure

185 Figure 3 shows the three-dimensional fluid flow paths on the smooth and rough fracture surfaces. Flow paths in all models are channelized by asperity contacts (i.e., preferential flow paths). As the fracture 186 187 aperture closes, both the flow velocity and the number of preferential flow paths decrease. Permeability in 188 each model was calculated from these simulated flow velocities for comparison with the experimental 189 results (Fig. 4). Our digital fracture simulations closely reproduced our experimental results for the smooth 190 (Fig. 4a) and rough (Fig. 4b) surfaces. Plots of the logarithmic permeability against stress show a change 191 with increasing effective normal stress from curving trends to linear trends. Figures 4c and 4d show 192 representative simulation results for the distribution of apertures (in grayscale) and associated flow rates 193 (in color online) through the smooth and rough fractures, respectively. Note that the flows in Fig. 4 represent 194 the vertically summed flow rates (perpendicular to the fracture plane) so that the three-dimensional flows in rough fracture walls can be projected on the x-y plane. These flows are then normalized with respect to 195 196 their maximum value, and regions with >1% of the maximum flow rate are visualized to accentuate the dominant flow paths. At low stresses, preferential flow paths form that cover most of the area with open 197 198 (non-zero) apertures (images i in Fig. 4). Isolated apertures also form, few at first, that are surrounded by 199 contacting asperities (zero aperture points), where the fluid is stagnant (white patches in Fig. 4c, d).

200 As stress increases, larger fractions of the fracture surfaces are in contact, and hence the dominant 201 flow paths decrease in number. As the dominant flow paths become less significant, the flow paths from the inlet to outlet are progressively disconnected (images iii and iv in Fig. 4). Accordingly, the permeability-202 203 stress relationship includes a transition: logarithmic permeability changes exponentially with stress while 204 the flow paths are connected (images i and ii) and linearly while the flow paths are disconnected (images iii and iv). The stress level where this change occurs can be defined as the hydraulic percolation threshold 205 $\sigma_{\rm HPT}$, which signifies the creation of continuous flow paths through rocks (Guéguen et al. 1997; Kirkby et 206 207 al. 2016). Roughness does not appear to greatly affect this threshold (see Fig. 5a). Interestingly, the disconnection of dominant electrical flow paths coincides with that of the fluid flow paths (Fig. 12 in 208

Appendix 3), even though electrical flow is spread more diffusely over the fracture than fluid flow (Fig. 13

in Appendix 3). Note that both hydraulic and electrical flow do not pass through the matrix owing to its

211 negligibly low permeability and electrical conductivity.

212 3.2 Effect of Stress and Asperity Contact

We present the evolution of several rock properties with stress changes in Fig. 5. Note that we discuss only P-wave velocity here as P- and S-wave velocities show similar tendencies (see Fig. 5c). Permeability and resistivity show a linear trend at stresses higher than σ_{HPT} but deviate from a linear trend at lower stresses, and neither property displays any dependence on fracture roughness (Fig. 5a, b). Elastic wave velocity varies notably with roughness, and unlike the case with porous rocks, there is no clear correlation between velocity and porosity; even at the same porosity (for example, ~1.2%), P- and S-wave velocities show variations (Fig. 5c, d).

220 Contact area increases with increasing stress, and hence the hydro-mechanical properties vary likewise (Jaeger et al. 2007; Wang and Cardenas 2016), therefore we examined the effect of contact area 221 222 on rock properties (Fig. 6). Permeability and resistivity are strongly correlated with contact area and 223 insensitive to roughness (Fig. 6a, b). Previous research has explored the relationship between fracture 224 permeability and contact area in synthetic fractures with identical mean aperture (Zimmerman et al. 1992). 225 Our results, from natural rock fractures with different apertures, also support a stable relationship between permeability and contact area. In contrast, elastic wave velocity is not a single function of contact area, 226 particularly when contact area is larger; instead, velocity generally increases with roughness (Fig. 6c). 227 228 Although porosity may partially contribute to this velocity variation (Fig. 5c, d), the correlation between 229 them appears to be weak. Another difference arising from the different roughness characteristics is the size variation of fracture asperity contacts. Figure 7 shows the distribution of contacting asperities along with 230 231 their size (in color online) in the smooth and rough fractures. Although both fractures have almost the same 232 contact area ($\sim 28\%$), the rough fracture contains larger asperities than the smooth fracture, and the contact 233 area in the smooth fracture consists mostly of small asperities. This difference in spatial distribution of the 234 asperities also produces the velocity difference. The effect of the asperity distribution on the velocity is small when the contact area is low, as contacting asperities in both fracture surfaces are few and small under 235 these conditions. 236

237 3.3 Relations of Hydraulic and Geophysical Properties

We examine the initial hypothesis of the link between hydraulic-electrical-elastic properties in the two plots of Fig. 8. The relationship of P-wave velocity with logarithmic permeability is sensitive to roughness, whereas resistivity clearly shows a simple relationship with permeability on a log-log basis that 241 does not vary with roughness (Fig. 8a). The relationship between logarithmic resistivity and flow area (the 242 areal fraction of preferential flow paths, i.e., the colored areas in Fig. 4c-d) is also insensitive to roughness 243 (Fig. 8b), reflecting the positive correlation between permeability and flow area (Watanabe et al. 2009; Nemoto et al. 2009). The relationship between P-wave velocity and flow area is roughness-dependent when 244 flow areas are below 60% but not so when flow areas exceed 60% (Fig. 8b). This roughness-independent 245 relationship between velocity and flow area at flow areas >60% arises from the roughness independence of 246 velocity in the fracture with lower asperity contacts (Fig. 6). The transition at ~60% flow area coincides 247 248 with the mechanical percolation threshold, as discussed below.

249 4 Discussion

250 4.1 Effect of Roughness on Rock Properties

251 We observe that all rock properties change markedly at elevated stresses that increase fracture asperity contacts. Changes in permeability and resistivity with stress (or contact area) are insensitive to 252 253 roughness, whereas the change in velocity with stress varies notably with roughness. Permeability and 254 resistivity are generally sensitive to pore connectivity (Walsh and Brace 1984; Guéguen and Palciauskas 1994), and hence their roughness-independent tendencies may imply that connectivity is unlikely to change 255 256 with differences in roughness even at the same stress. Although detailed investigations with various samples 257 are needed to assess the correlation of connectivity with these transport properties, the close similarity of the percolation threshold $\sigma_{\rm HPT}$ in different roughness models also supports our hypothesis. The theoretical 258 259 study of Zimmerman et al. (1992) shows that transport properties are strongly dependent on the contact 260 area and less sensitive to the microstructure. Because the contact area of different roughness models is 261 almost the same under similar stress conditions in our mated fracture (Table 2), the roughness independence of transport properties in our results may be related to the roughness independence of the contact area. The 262 263 roughness-independent relationship between resistivity and permeability (Fig. 8) also suggests that at least the mechanisms underlying changes in both properties are the same and do not depend on roughness. Note 264 265 that such roughness independences may be limited to mated fractures, as the contact area of sheared fractures may be found to change with roughness. The slope of the resistivity-permeability relationship is 266 related to the tortuosity of the pore structure (Brown 1989). The smaller change in resistivity at higher 267 ranges of permeability $(>10^{-11})$ indicates that tortuosity also changes relatively little, whereas the larger 268 change in resistivity at lower permeability ranges ($<10^{-11}$) implies that tortuosity responds dramatically to 269 aperture closure. This change in slope marks a transition of the flow pattern. At higher permeabilities 270 271 (images i in Fig. 4), flow paths are largely channelized and the flow area is sufficient (>60%), whereas at 272 lower permeabilities (images ii–iv in Fig. 4), flow paths are sinuous (or have fewer connected channels).

273 The roughness dependence of the velocity change arises mainly from differences in porosity and 274 contact area, velocity being higher in samples with lower porosity or larger contact area even at the same stress condition (Figs. 5 and 6). In addition, different roughness characteristics produce size variations of 275 the fracture asperity contacts, which also affect the velocity difference (Fig. 7). On one hand, larger contact 276 sizes generally contribute to stiffening the rock (Guéguen and Boutéca 2004), and hence elastic energy 277 propagates dominantly in the larger asperity due to its high bond energy. On the other hand, a large number 278 279 of small asperities reduces the bulk elastic stiffness. In the case of cracked materials, thin cracks (i.e., 280 smaller aspect ratio) reduce the bulk elastic energy more than stiff cracks (i.e., lower aspect ratio) even at 281 the same volume, because the stress strongly concentrates on the edges of thin cracks rather than those of 282 stiff cracks (e.g., Budiansky and O'connell, 1976; Kachanov, 1994). Similarly, our fractured sample also 283 shows a stress concentration on small asperities that are dominant in smooth fractures (Appendix 4). 284 Therefore, we infer that the velocity difference (Fig. 6) may also arise from the size variation of contacting 285 asperities. Figure 9 depicts our conceptual model of roughness-induced variation of asperity contacts and possible changes in velocity. Aperture closure with increasing stress enlarges asperity contacts, and hence 286 287 the velocity increases in both smooth and rough fractures (Fig. 6). Under higher stress conditions (Fig. 9b and d), even at the same stress and similar proportions of contact area, the asperity size differs due to the 288 289 roughness, and thus the roughness dependency of velocity is especially marked at higher contact areas. This 290 effect of asperity size is small when the contact area is low, as contacting asperities are few and small under 291 these conditions. Because our results also incorporate the porosity effect, further study is needed to confirm 292 the effect of asperity distribution on velocity by investigations of various natural fractures having identical porosity. It may be of interest that permeability and resistivity do not vary with the size and distribution of 293 asperities because they are integrated properties (Zimmerman et al. 1992), which are insensitive to the 294 295 microscopic structure but sensitive to the macroscopic structure (i.e., contact area).

4.2 Transitions in the Fracture Flow Pattern and Associated Changes in Geophysical Properties

Although many experimental studies in intact rocks have revealed the evolution of rock properties with stress change (Brace and Orange 1968; Scholz 2002; Paterson and Wong 2005), some observations have detected unusual changes of rock properties that cannot be explained by these experimental results (Park 1991; Xue et al. 2013). The presence of mesoscale fractures may account for these discrepancies. To investigate this issue, we compiled our results on the evolution of rock properties in single fractures and compared them with the changes in flow rate distribution within the fracture. These changes in rock properties can be categorized as roughness-dependent (Fig. 10a) or roughness-independent (Fig. 10b).

Elastic wave velocity and flow area are both roughness-dependent, thus we can distinguish separate mechanical percolation thresholds for smooth fractures (σ'_{MPT}) and rough fractures (σ'_{MPT}), defined in both

cases as the stress at which velocity reaches 90% of its maximum value (Fig. 10a). Because σ_{MPT} is smaller 306 307 than σ'_{MPT} , velocity increases more sharply with stress in smooth fractures than in rough fractures. The difference arises from a discrepancy in the heterogeneous aperture distribution (Fig. 1c). In cracked rock 308 309 samples, it is well known that a rapid velocity increase with stress implies the closure of a dominant set of 310 cracks with a similar aspect ratio (i.e., a sharp bend in the aspect ratio spectrum), whereas a monotonic increase results from closure of cracks of various aspect ratios (i.e., a broader bend in the aspect ratio 311 312 spectrum) (Tsuji et al. 2008; Mavko et al. 2009). By analogy with this model, a more rapid velocity increase with stress in the smooth fracture may reflect a biased distribution of aperture sizes such that velocity 313 increases rapidly with the closure of apertures of the dominant size and changes only slightly afterward. 314 315 Resistivity and permeability are both roughness independent (Fig. 10b). Tendencies of these changes depend on the hydraulic percolation threshold σ_{HPT} , which is higher than σ_{MPT} (Guéguen et al. 1997). 316

Figure 9c schematically illustrates these changes in rock properties as three stages (Stage I to Stage 317 318 III) defined by transitions of the fracture flow pattern within a subsurface fracture with increasing stress. At lower stresses, Stage I represents aperture-dependent flow, where fluid flows within most of the void 319 320 space (the aperture) and the flow area decreases as the mean aperture decreases (Fig. 10a). This stage is typified by largely connected flow paths and sufficient flow area, in which tortuosity is insensitive to stress 321 322 changes. All rock properties change rapidly with increasing stress in this stage. Stage II, at stresses higher 323 than σ_{MPT} but lower than σ_{HPT} , represents *aperture-independent flow*, in which isolated apertures appear 324 and become areas without flow. In this stage, tortuosity becomes sensitive to stress change, connected 325 channels decrease, and as a result flow area decreases markedly with increasing stress. Unlike Stage I, the rate of decrease in flow area exceeds the decrease in mean aperture size (Fig. 10a), suggesting that the 326 fracture flow at this stage is not fully characterized by aperture size, but instead is controlled by asperity 327 328 contacts. Although elastic wave velocity remains nearly constant with rising stress, permeability and resistivity change exponentially because flow paths are still connected and thus these attributes are less 329 330 sensitive to the spatial distribution of asperity contacts. In Stage III, at stresses higher than σ_{HPT} , flow paths become disconnected and result in disconnected flow. In this stage, logarithmic permeability and resistivity 331 332 change linearly with stress, and areas without flow become a significant fraction of the fracture area. Because Stage II begins when the velocity ceases to change with rising stress, the transition from Stage I 333 334 to II can be detected by velocity monitoring, whereas resistivity is sensitive to the transition from Stage II 335 to III. This means that, if monitoring detects the combination of almost constant velocity and exponential 336 change in the logarithmic resistivity, it may signal the presence of aperture-independent (Stage II) flow.

If crustal stress can be considered constant (i.e., on relatively short timescales), then changes in the
 fracture flow pattern with changes in effective normal stress represent changes in pore pressure. This

finding may show promise in two applications. One application involves the evolution of fluid flow along 339 340 faults, which is part of the fault reactivation cycle triggered by pore pressure perturbations. Our model of Stage I reproduces observations of high permeability (Xue et al. 2013; Kinoshita et al. 2015), low resistivity 341 (Mazzella and Morrison 1974; Park 1991) and low seismic velocity (Brenguier et al. 2008; Taira et al. 342 2018) resulting from high pore pressures associated with earthquakes. The changes in elastic wave velocity 343 and permeability from Stage I to II (Fig. 10a, b) are in good agreement with observations after earthquakes 344 (Xue et al. 2013; Nimiya et al. 2017). Under Stage II conditions, a resistivity change of ~10–20% (Park 345 1991) corresponds to a stress perturbation of 0.2-1.4 MPa, and a permeability change of ~30-40% (Xue et 346 al. 2013) corresponds to a stress perturbation of 0.9–3.2 MPa. Moreover, during Stage II, seismic velocity 347 is nearly constant after healing stabilizes the mechanical properties of faults (Nimiya et al. 2017). 348 349 Nevertheless, subsurface fracture flow could be changing because our results show that seismic velocity is 350 insensitive to pressure above σ_{MPT} . Fault healing eventually leads to large areas of little or no flow (Stages II and III), where mineral precipitation is favored. Pore pressure changes following earthquakes, triggered 351 by several mechanisms such as mineral precipitation (Sibson 1992; Tenthorey et al. 2003), lead rapidly to 352 353 decreases in seismic velocity, increases in permeability and decreases in resistivity, after which all of these 354 properties recover (Mazzella and Morrison 1974; Xue et al. 2013; Taira et al. 2018), which suggests that fracture flow patterns return to their initial condition (Stage I). Thus our inferred transitions in the fracture 355 flow pattern may explain how the cycle of earthquake recurrence is correlated with geophysical 356 357 observations, complementing the fault-valve model (Sibson et al. 1988).

358 The other application involves the changes in productivity of fluid resources in fractured reservoirs 359 (for example, geothermal reservoirs) during development. Because increased elastic wave velocity 360 coincides with decreased permeability during Stage I, a gradual velocity increase in geothermal fields 361 implies a slight decrease in reservoir permeability (Taira et al. 2018). If a point is reached where velocity 362 remains steady while resistivity decreases, the fracture flow pattern would be at Stage II or III, where the flow area shrinks considerably. A limited flow area could lead to poorer thermal performance during a 363 364 geothermal development (Hawkins et al. 2018) and could lower reservoir permeability by as much as two 365 orders of magnitude (Fig. 9).

To apply our results to real field locations, we need to consider the scale dependencies of rock properties. For example, although longer fracture lengths generally mean higher roughness values (Brown and Scholz 1985; Power et al. 1987; Power and Durham 1997; Jaeger et al. 2007), fracture permeability in joints is only partially dependent on fracture length (Ishibashi et al. 2015). This suggests that roughnessindependent properties, including resistivity (Fig. 10b), may have a weak dependence on fracture length, thus resistivity monitoring could be effective for detecting changes in hydraulic properties at field scale.

On the other hand, elastic wave velocity is a roughness-dependent property (Fig. 10a) and thus varies with the fracture scale. However, this scaling effect on velocity can be modified by considering the ratio of the wavelength and the fracture length (Mavko et al. 2009). Although our study adopted a zero-frequency assumption for the velocity calculation, the scaling effect on velocity can be addressed by considering finite wavelengths. Because finite-difference time-domain modeling of wave fields in fractured media requires more complex assumptions, such as fracture compliance (Bakulin et al. 2000; Minato and Ghose 2016; Pyrak-Nolte et al. 1990), the scale dependency on velocity needs to be further explored.

379 4 Conclusions

380 We investigated the correlated changes in fracture permeability, flow area, resistivity and elastic wave velocity of joints under increasing normal stress by coupling experimental data with digital fracture 381 simulations. We found that changes in permeability and resistivity are controlled by fluid connectivity, 382 383 which is more dependent on stress than on fracture roughness. The relationship between hydraulic and 384 electrical properties is independent of roughness, owing to the roughness independence of fluid connectivity 385 (as expressed by the hydraulic percolation threshold). The roughness dependence of elastic wave velocity arises from spatial distributions of contacting asperities as well as the roughness dependency of porosity. 386 387 These relationships show promise for improving geophysical interpretations. Our lattice Boltzmann fluid flow simulation revealed that the fracture flow pattern undergoes transitions through three stages as 388 389 effective normal stress increases: aperture-dependent flow (Stage I), aperture-independent flow (Stage II) 390 and disconnected flow (Stage III). Elastic wave velocity may be a useful indicator of the Stage I-II 391 transition, and resistivity may be a sensitive indicator of the Stage II-III transition. The relationships we 392 have revealed may enable geological regimes associated with stress changes, such as seismogenic zones 393 and geothermal reservoirs, to be monitored remotely on the basis of their geophysical properties.

394

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578	

579 Figures and Tables

580 **Table 1**. Physical properties used for finite-element modeling of resistivity and elastic wave velocity.

581

		Conductivity [mS/m]	Bulk modulus [GPa]	Shear modulus [GPa]	Density [kg/m ³]			
	Solid	0.01^{*}	58.6***	31.0***	2650			
	Fluid	5000**	2.25	0	994			
	* Based on the experimental result of the resistivity measurement under dry condition							
	** For seawater							
1	*** Based on P- and S-wave velocity measurements under dry conditions and high confining							
1	pressure (200 MPa)							

585 586

582 583 584



587



1.0

Aperture (mm)

1.2

1.4

1.6

1.8

2.0

0∟ 0.2

0.4

0.6

0.8



Fig. 2 Model setup of the 3D digital fracture simulation. Fluid flow and applied voltage are defined as parallel to the fracture plane, whereas elastic wave velocity is defined as perpendicular to the fracture plane.

593 Both the lattice Boltzmann simulation and finite-element modeling adopt a periodic boundary condition

594

595



Fig. 3 Three-dimensional flow paths calculated by the lattice Boltzmann simulation on the surface of the (a–c) smooth and (d–f) rough fracture under various effective normal stress (σ_{eff}). Flow velocity (in color online) is illustrated on the footwall of each fracture surface



600

Fig. 4 Experimental and simulated fracture permeabilities with increasing effective normal stress of the (a) 601 smooth and (b) rough fractures and representative images derived from the simulation showing fracture 602 flow distribution (color) within the heterogeneous aperture distribution (grayscale) with aperture closure of 603 the (c) smooth and (d) rough fractures. Black and white diamonds in (a) and (b) represent experimental and 604 605 simulated results, respectively. Red diamonds in (a) and (b) are the representative results that are illustrated in (c) and (d). The normalized flow in (c) and (d) represents the vertical summed flow, normalized by the 606 maximum value in each condition, and the regions with <1% of the maximum flow rate are colorless (color 607 608 figure online). Dashed red ellipses in (c) and (d) show regions that are disconnected from the dominant flow 609 paths



611 **Fig. 5** Graphs showing changes in (a) permeability, (b) resistivity, (c) elastic wave velocity and (d) porosity 612 in relation to effective normal stress. Dashed lines are extrapolations from the data in the regions of 613 disconnected flow (gray), as defined by the value of σ_{HPT} . Gray symbols in (c) and (d) (green in the online 614 version) represent pairs of data points that have comparable porosity (~1.2%)



Fig. 6 Graphs showing changes in (a) permeability, (b) resistivity, and (c) elastic wave velocity in relation to the contact area. Gray symbols in (c) (green in the online version) represent pairs of data points with comparable contact area (~28%), whose asperity distributions are shown in Fig. 7



Fig. 7 Distribution of asperity contacts on the (a) smooth and (b) rough fractures, both of which have a contact area of ~28%. Color represents the asperity size (color figure online)



Fig. 8 Graphs showing correlations between (a) permeability and geophysical properties and (b) flow area and geophysical properties. Orange diamonds and green circles (color online) represent resistivity and P-wave velocity, respectively, and open and solid symbols represent smooth and rough fractures, respectively



- 629 Fig. 9 Schematic images of the voxel model of the fracture aperture structure and asperity contacts, showing
- 630 their changes with stress for (a–b) smooth and (c–d) rough fractures. The apertures are shown in blue,
- 631 matrix in gray, and contacting asperities as black solid boxes (color figure online)



Fig. 10 Schematic diagram of changes with respect to pressure in (a) roughness-dependent properties and (b) roughness-independent properties and (c) schematic images of the three-stage transition of fracture flow patterns. All rock physical properties in (a) and (b) are normalized based on our results. Gray lines in (a) represent mechanical percolation thresholds σ_{MPT} and σ'_{MPT} of smooth and rough fractures, respectively, which distinguish aperture-dependent and aperture-independent flows (Stages I and II). The gray line in (b) represents the hydraulic percolation threshold σ_{HPT} , which represents the boundary between connected flow (Stages I and II) and disconnected flow (Stage III)

641 Appendix 1. Effect of Voxel Size

The voxel size potentially affects the absolute value of permeability and resistivity because these quantities 642 are sensitive to the connectivity of the local aperture. To check this possible effect of voxel size, we 643 analyzed the permeability and resistivity of models with different voxel sizes, preparing 48 mm \times 48 mm 644 fracture models from the rough fracture surfaces using cubic systems with 0.05 mm, 0.1 mm, and 0.2 mm 645 voxels. Figure 11 plots the permeability and resistivity against the contact area from the models of each 646 647 voxel size. Although voxel size affects permeability to some degree, the maximum difference between the results with 0.05 mm and 0.1 mm voxels is less than half an order of magnitude (Fig. 11a). The difference 648 in resistivity is much smaller (Fig. 11b). Notably, the models with 0.05 mm and 0.1 mm voxel sizes show 649 similar trends in both cases of permeability and resistivity. Because the computational cost is prohibitive at 650 our original fracture size (48 mm \times 72 mm) in a 0.1 mm cubic system, we conclude that the 0.1 mm voxel 651 652 size is suitable for our qualitative interpretations of permeability and resistivity.







Fig. 11 Graphs showing (a) permeability and (b) resistivity with different sizes of voxel. Open diamonds, solid diamonds, and open circles represent the results from 0.2, 0.1, and 0.05 mm voxel sizes, respectively

659 Appendix 2. Supplementary Material

Table 2 summarizes the simulation results. Movie files of the lattice Boltzmann simulations can be found online.

662

663

Table 2. Summary of simulation results: σ_{eff} is the effective normal stress, φ is the porosity, k_f is the fracture permeability, and ρ is the electrical resistivity. The smooth fracture at $\sigma_{eff} = 12.7$ MPa and the rough fracture at $\sigma_{eff} = 4.76$ MPa have similar porosity (1.19%), but differ in velocity (V_p) by 0.11 km/s. Similarly, the smooth fracture at $\sigma_{eff} = 2.87$ MPa and the rough fracture at $\sigma_{eff} = 3.79$ MPa have similar contact area (~28%), but differ in velocity (V_p) by 0.32 km/s

669 670

σ_{eff}	Mean aperture	Contact area	arphi	log ₁₀ k _f	Flow area	$\log_{10} ho$	V_p	V_{s}	
[MPa]	[mm]	[%]	[%]	[m²]	[%]	[Ω m]	[km/s]	[km/s]	
Smooth fracture									
0.165	0.238	1.66	2.91	-10.4	95.3	1.16	4.12	2.30	
0.654	0.178	9.41	2.20	-10.8	77.4	1.44	4.42	2.79	
1.14	0.158	15.4	1.96	-11.0	61.0	1.63	4.54	2.88	
2.07	0.139	23.4	1.73	-11.3	49.4	1.88	4.67	2.97	
2.87	0.130	28.1	1.61	-11.4	41.2	2.04	4.74	3.01	
4.05	0.121	33.0	1.50	-11.5	32.8	2.18	4.80	3.05	
5.83	0.112	38.2	1.39	-11.6	25.0	2.39	4.87	3.09	
8.54	0.103	43.5	1.28	-11.8	19.3	2.53	4.95	3.12	
12.7	0.0951	48.6	1.19	-11.9	12.4	2.67	5.01	3.15	
19.1	0.0873	53.5	1.10	-12.1	7.12	2.82	5.08	3.17	
28.7	0.0801	58.0	1.01	-12.3	5.57	3.06	5.14	3.19	
			R	ough fractu	re				
0.208	0.330	2.91	2.32	-10.2	87.6	1.30	4.20	2.37	
1.07	0.233	13.4	1.64	-10.8	59.8	1.57	4.66	2.91	
3.02	0.187	25.2	1.32	-11.2	43.3	2.00	5.01	3.12	
3.79	0.178	28.3	1.25	-11.3	34.4	2.11	5.06	3.15	
4.76	0.169	31.5	1.19	-11.4	33.6	2.20	5.12	3.17	
6.00	0.161	34.9	1.14	-11.5	33.6	2.27	5.16	3.19	
7.55	0.154	38.4	1.09	-11.6	25.8	2.36	5.20	3.21	
9.55	0.146	42.0	1.04	-11.7	26.6	2.45	5.24	3.22	
12.1	0.139	45.6	0.984	-11.9	22.0	2.62	5.29	3.22	
15.5	0.132	49.4	0.933	-12.0	18.8	2.81	5.29	3.22	
20.0	0.125	53.2	0.884	-12.1	10.8	2.94	5.34	3.22	
25.9	0.118	57.0	0.836	-12.3	14.2	3.03	5.41	3.21	

673 Appendix 3. Local Electrical Flow

674 The local electrical flows are visualized in Fig. 12 in the same fashion as the fluid flow paths in Fig. 4 in the main text. The flow in Fig. 12 shows vertically summed electric currents (perpendicular to the fracture 675 plane), normalized with respect to their maximum value. Regions with >1% of the maximum electric 676 current are visualized to accentuate the dominant paths. Although the trend of transient changes of electrical 677 flow with aperture closure is similar to that of fluid flow, electrical flow is spread more diffusely over the 678 679 fracture than fluid flow (Brown 1989). From these results, the conductive area is calculated, defined as the 680 ratio of the area of dominant electrical flow paths to the area of the fracture plane (colored area in Fig. 12). Figure 13, which plots the evolution of both the conductive area and flow area at elevated stress, clearly 681 shows that conductive area is slightly greater than flow area in both smooth and rough fractures. It is notable 682 that the disconnection of dominant electrical flow paths coincides with that of the fluid flow paths (i.e., 683 684 hydraulic percolation threshold).





Fig. 12 Local electrical flow distribution (color) within the heterogeneous aperture distribution (grayscale) with aperture closure of the (a) smooth and (b) rough fractures. Images i–iv are representative results at the same stress conditions as in Fig. 4. The normalized electric current represents the vertical summed electric current, normalized by the maximum value in each condition, and the regions with <1% of the maximum electric current are colorless (color figure online). Dashed red ellipse shows a regions that are disconnected from the dominant paths</p>



Fig. 13 Graphs showing changes in flow area (blue symbol) and conductive area (orange symbol) in relation
 to effective normal stress. Open and closed diamonds show the results from smooth and rough fractures,

695 to effective a696 respectively

698 Appendix 4. Stress Concentration on Small Asperities

To reveal the effect of asperity size on the stress concentration, we visualized the local distribution of stress perpendicular to the fracture plane (σ_3). Figures 14 shows the distribution of σ_3 at the same condition as Fig. 7 (contact area ~28%) in smooth and rough fractures. The stress value is normalized by its average and visualized only in asperities. In both cases, the stress concentrates strongly on smaller asperities, whereas the stress across larger asperities is relatively small. Smaller asperities are more dominant in the smooth fracture case (Fig. 14c).





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Fig. 14 Local distribution of stress (color) across the fracture plane in contact areas at the same condition as Fig. 7 (contact area ~28%) of the (a) smooth and (b) rough fractures and (c) histogram of asperity size

in the smooth (black) and rough (white) fractures (color figure online)