

# Turbulent Mixing in the Benthic Biolayer of Streams

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## Abstract

In this paper we develop and test a rigorous modeling framework, based on Duhamel’s Theorem, for the unsteady one-dimensional transport and mixing of a solute across a flat sediment-water interface (SWI) and through the benthic biolayer of a turbulent stream. The modeling framework is novel in that it allows for depth-varying diffusivity profiles, accounts for the change in porosity across the SWI and captures the two-way coupling between evolving solute concentrations in both the overlying water column and interstitial fluids of the sediment bed. We apply this new modeling framework to an extensive set of previously published laboratory measurements of turbulent mixing across a flat sediment bed, with the goal of evaluating four diffusivity profiles (constant, exponentially declining, and two hybrid models that account for molecular diffusion and enhanced turbulent mixing in the surficial portion of the bed). The exponentially declining profile is superior (based on RMSE, coefficient of determination, AICc, and model parsimony) and its reference diffusivity scales with a dimensionless measure of stream turbulence and streambed permeability called the Permeability Reynolds Number, . The diffusivity’s dependence on changes abruptly at , reflecting different modes of mixing below (dispersion) and above (turbulent diffusion) this threshold value. The depth-scale over which the diffusivity exponentially decays is about equal to the thickness of the benthic biolayer (2 to 5 cm), implying that turbulent mixing, and specifically turbulent pumping, may play an outsized role in the biogeochemical processing of nutrients and other contaminants in stream and coastal sediments.

# Vertical Structure and Scaling of Turbulent Mixing in the Benthic Biolayer of Stream and Coastal Sediments

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## Key Points:

- A one-dimensional diffusion model, based on Duhamel's Theorem, is developed for turbulent mass transport in the benthic biolayer
- The model reproduces solute transfer above and below the sediment-water interface provided the diffusivity decays exponentially with depth
- Turbulent mixing increases with the Permeability Reynolds Number, and appears to be driven by turbulent pumping

## Abstract

In this paper we develop and test a rigorous modeling framework, based on Duhamel's Theorem, for the unsteady one-dimensional transport and mixing of a solute across a flat sediment-water interface (SWI) and through the benthic biolayer of a turbulent stream. The modeling framework is novel in that it allows for depth-varying diffusivity profiles, accounts for the change in porosity across the SWI and captures the two-way coupling between evolving solute concentrations in both the overlying water column and interstitial fluids of the sediment bed. We apply this new modeling framework to an extensive set of previously published laboratory measurements of turbulent mixing across a flat sediment bed, with the goal of evaluating four diffusivity profiles (constant, exponentially declining, and two hybrid models that account for molecular diffusion and enhanced turbulent mixing in the surficial portion of the bed). The exponentially declining profile is superior (based on RMSE, coefficient of determination, AICc, and model parsimony) and its reference diffusivity scales with a dimensionless measure of stream turbulence and streambed permeability called the Permeability Reynolds Number,  $Re_K$ . The diffusivity's dependence on  $Re_K$  changes abruptly at  $Re_K = 1$ , reflecting different modes of mixing below (dispersion) and above (turbulent diffusion) this threshold value. The depth-scale over which the diffusivity exponentially decays is about equal to the thickness of the benthic biolayer (2 to 5 cm), implying that turbulent mixing, and specifically turbulent pumping, may play an outsized role in the biogeochemical processing of nutrients and other contaminants in stream and coastal sediments.

54 **Plain Language Summary**

55 How far and fast pollutants travel downstream is often conditioned on what happens in a  
56 thin veneer of biologically active bottom sediments called the benthic biolayer. However,  
57 before a pollutant can be removed in the benthic biolayer it must first be transported  
58 across the sediment-water interface and through the interstitial fluids of these surficial  
59 sediments. In this paper we investigate, through mathematical modeling and an  
60 evaluation of previously published experimental data, the role that water column  
61 turbulence plays in transporting solutes into and through the benthic biolayer of streams  
62 and coastal sediments.

63

## 1 Introduction

Many physical and biological processes in aquatic ecosystems depend on, or are strongly affected by, turbulent fluid motions at the sediment-water interface (SWI) (Franca and Brocchini, 2015; Grant and Marusic, 2011). Stream turbulence drives the vertical transport of dissolved constituents through the water column (Tomasek et al., 2018; O'Connor and Hondzo, 2008; Hondzo, 1998) imposing an upper limit on the rate that reactive constituents, nitrate for example, can be assimilated and removed by the streambed (Grant et al., 2018a). Stream turbulence also facilitates the transport and mixing of dissolved and fine particulate materials and energy across the SWI and in the benthic biolayer, the upper 5 cm of the streambed where much of the microbial biomass, as well as nutrient and pollutant processing, is concentrated (Tomasek et al., 2018; Knapp et al., 2017; Caruso et al., 2017; Trauth et al., 2014; Harvey et al., 2013; Zarnetske et al., 2011; Battin et al., 2008; Dahm et al., 2002).

At the scale of the benthic biolayer, stream turbulence facilitates mixing in at least two ways (**Figure 1a**): (1) “turbulent pumping” occurs when spatially coherent eddies spawn pressure waves that travel along the SWI and drive temporally oscillating laminar flow across the interface (Kim et al., 2020; Zhong et al., 2016; Boano et al., 2011; Higashino et al., 2009); and (2) “turbulence penetration” occurs when turbulent eddies drive intermittent transport of mass and momentum across the interface (Kim et al., 2020; Roche et al., 2018; Voermans et al., 2017; Reidenbach et al., 2010; Packman et al., 2004). If ripples and dunes are present on the streambed surface, mixing across the benthic biolayer is also facilitated by the advective transport of solutes across the SWI in spatially isolated upwelling and downwelling zones, “bedform pumping” (Azizian et al., 2018;

Grant et al., 2014; Fleckenstein et al., 2010; Cardenas et al., 2008; Elliot and Brooks, 1997a,b; Thibodeaux and Boyle, 1987)) and the entrapment and release of interstitial fluids associated with bedform migration, “bedform turnover” (Wolke et al., 2020; Zheng et al., 2019; Elliot and Brooks, 1997a,b)). Solute mixing in the streambed is also controlled by molecular diffusion (which smooths out the steep concentration gradients generated by the above transport mechanisms, Hester et al., 2017), bio-diffusion (in which pore fluids and sediment are “pumped” through benthic macrofauna and plants, Thibodeaux et al., 2011), and the streambed’s permeability and porosity fields (which can vary temporally and spatially, Laube et al., 2018; Newcomer et al., 2016; Stewardson et al., 2016; Salehin et al., 2004; Herzog et al., 2018).

At present, only under highly idealized experimental or modeling conditions is it possible to resolve the spatially and temporally complex eddy advection pathways generated by the transport mechanisms described above. Due to its simplicity, mathematical tractability and prior agreement with scaling studies, a common alternative is to approximate mass transport across the SWI, and through the benthic biolayer, as a flux-gradient diffusive process (Voermans et al., 2018):

$$J(y,t) = -D_{\text{eff}}(y) \frac{\partial(\theta C_s)}{\partial y} \quad (1a)$$

Here,  $J(y,t)$  [ $\text{M L}^{-2} \text{T}^{-1}$ ] and  $C_s(y,t)$  [ $\text{M L}^{-3}$ ] are the horizontally and temporally averaged vertical flux and interstitial solute concentration at depth  $y$  [L] and time  $t$  [T] in the sediment. The effective diffusivity,  $D_{\text{eff}}$  [ $\text{L}^2 \text{T}^{-1}$ ], for solute mixing in the surficial sediments of a stream or coastal sediment sums over contributions from the tortuosity-modified molecular diffusion,  $D'_m$  [ $\text{L}^2 \text{T}^{-1}$ ], dispersion,  $D_d$  [ $\text{L}^2 \text{T}^{-1}$ ], and turbulent

diffusion,  $D_t$  [ $L^2 T^{-1}$ ] (Roch et al., 2019; Roche et al., 2018; Grant et al., 2018b; Voermans et al., 2018; Voermans et al., 2017; Chandler et al., 2016; Zhong et al., 2016; Boano et al., 2014; Grant et al., 2012; Boano et al., 2011; Grant and Marusic, 2011; Reidenbach et al., 2010; O'Connor and Harvey, 2008; O'Connor and Hondzo, 2008; Packman et al., 2004; Nagaoka and Ohgaki, 1990; Richardson and Parr, 1988):

$$D_{\text{eff}} = D'_m + D_d + D_t \quad (1b)$$

Dispersion arises from spatial correlations between the time-averaged vertical velocity component and the local mean solute concentration, while turbulent diffusion arises from temporal correlations between the turbulent vertical velocity component and the instantaneous turbulent concentration field (Voermans et al., 2018).

The use of equations (1a) and (1b) to describe mixing in the benthic biolayer raises three questions. First, given that the mean flow and shear (turbulence) fields responsible for mixing across the benthic biolayer are damped out with depth by viscous dissipation (He et al., 2019; Roche et al., 2018; Voermans et al., 2017; Pokrajac and Manes, 2009; Bruegem et al., 2006) how should the effective diffusivity be structured vertically, and does the answer depend on the nature of the transport mechanism under consideration (e.g., dispersion or turbulent diffusion)? Second, once an appropriate vertical structure is selected for the effective diffusivity, how well does the flux-gradient diffusive model (equation (1a)) represent solute transport through the streambed? Third, how do we extrapolate effective diffusivities measured in the laboratory to streams and coastal sediments? In this paper we focus on addressing these three questions in the context of turbulent mixing across a flat sediment bed. Complementary efforts are

underway to address mixing in the benthic biolayer by bedform pumping and bedform turnover.

The paper is organized as follows. In **Section 2** we demonstrate, through an application of Duhamel's Theorem (Perez et al., 2013), that solute concentration in the interstitial fluids of the sediment bed can be represented as the convolution of solute concentration in the water column and a Green's function for mass transport in the interstitial fluids of the streambed (Leij et al., 2000). This starting point leads to a set of explicit solutions, valid for any Green's function, for the spatiotemporal evolution of solute concentration in the water and sediment columns of a closed system. Notably, these solutions capture the two-way coupling of evolving solute concentrations above and below the SWI, whereby mass transfer out of the streambed alters mass concentration in the overlying water column which, in turn, alters mass transfer into the streambed, and so on (**Figure 1b**). We then derive six Green's functions for two choices of the lower boundary condition (finite or semi-infinite sediment domain) and four functional forms of the diffusivity depth profile. In **Section 3** we demonstrate how this theory can be used to simulate unsteady mass transfer in a stirred tank experiment, and in **Section 4** apply it to previously published measurements of turbulent mass transfer across a flat unconsolidated sediment bed in a well-stirred tank (Chandler et al., 2016; Chandler, 2012). We address the three questions raised above in **Section 5** and present our conclusions in **Section 6**.

[Figure 1 goes about here]

## **2 Analytical Modeling Framework**

### **2.1 Governing Equations for Turbulent Mixing in the Benthic Biolayer**



Double averaging over the turbulent timescale (e.g., estimated from the ratio of water depth and stream velocity (Pope, 2012)) and the horizontal plane of the SWI, and assuming that streambed porosity does not change appreciably through the benthic biolayer (i.e., the variable  $\theta$  appearing in equation (1a) is a fixed constant in the upper 5 cm or so of the streambed (Knapp et al., 2017)), we obtain the following mass conservation equation for one-dimensional diffusion of a solute through the interstitial fluids of the sediment bed:

$$\frac{\partial C_s}{\partial t} = \frac{\partial}{\partial y} \left( D_{\text{eff}}(y) \frac{\partial C_s}{\partial y} \right) \quad (2a)$$

Equation (2a) equates the accumulation of mass in a differential horizontal slice of the sediment beneath a turbulent stream (left hand side) to the vertical diffusive transport (right hand side) of a conservative (non-reactive and non-adsorbing) solute (Incropera et al., 2007). The coordinate  $y$  increases with depth into the streambed and its origin (at  $y=0$ ) is positioned at the horizontal plane of the SWI (**Figure 1a**).

In this study we explore four functional forms of the effective diffusivity depth profile,  $D_{\text{eff}}(y) = D_{\text{eff},0} f(y)$ , where the variable  $D_{\text{eff},0}$  is the surficial effective diffusivity (at the SWI,  $y=0$ ) and  $f(y)$  (unitless) is a piecewise continuous function that equals unity at the SWI (i.e.,  $f(0)=1$ ). After substituting this functional form for the effective diffusivity, equation (2a) can be rewritten in dimensionless form where the new dependent variable,  $c_s$  [-], incorporates the solute's initial concentration in the interstitial pore fluids of the sediment bed and the overlying water column ( $C_{s0}$  and  $C_{w0}$  [ $\text{M L}^{-3}$ ], respectively):

$$174 \quad \frac{\partial c_s}{\partial \bar{t}} = \frac{\partial}{\partial \bar{y}} \left( f(\bar{y}) \frac{\partial c_s}{\partial \bar{y}} \right) \quad (2b)$$

$$175 \quad c_s(\bar{y}, \bar{t}) = \frac{C_s(\bar{y}, \bar{t}) - C_{s0}}{C_{w0} - C_{s0}}, \quad c_s(\bar{y}, \bar{t}) \in [0, 1] \quad (2c)$$

$$176 \quad \bar{t} = t/t_T \geq 0, \quad t_T = 1/(D_{\text{eff},0} a^2), \quad \bar{y} = ay \geq 0 \quad (2d)$$

177 The constant  $a$  [ $L^{-1}$ ] is an inverse depth-scale (the definition of which depends on the  
 178 choice of diffusivity profile, see later) and  $t_T$  is a time constant for solute mixing in the  
 179 benthic biolayer. Given the definition of the dimensionless concentration group (equation  
 180 (2c)), the initial condition for equation (2b) becomes:

$$181 \quad c_s(\bar{y}, \bar{t} = 0) = 0 \quad (3a)$$

182 At the upper boundary (at the SWI,  $y=0$ ) we require that the interstitial tracer  
 183 concentration equals the tracer concentration in the overlying water column,  
 184  $C_s(y=0, t) = C_w(t)$  [ $M L^{-3}$ ], which depends only on time; i.e., solute concentration in the  
 185 water column is well-mixed. Expressed in dimensionless form the upper boundary  
 186 condition becomes:

$$187 \quad c_s(\bar{y}=0, \bar{t}) = \frac{C_w(\bar{t}) - C_{s0}}{C_{w0} - C_{s0}} H(\bar{t}) = c_w(\bar{t}) H(\bar{t}) \quad (3b)$$

$$188 \quad c_w(\bar{t}) = \frac{C_w(\bar{t}) - C_{s0}}{C_{w0} - C_{s0}}, \quad c_w(\bar{t}) \in [0, 1], \quad H(\bar{t}) = \begin{cases} 0, & \bar{t} < 0 \\ 1, & \bar{t} > 0 \end{cases} \quad (3c)$$

189 The Heaviside function  $H(\bar{t})$  [-] appearing on the righthand side of equation (3b) ensures  
 190 that the upper boundary condition is zero for  $\bar{t} < 0$  (this detail becomes important for the  
 191 application of Duhamel's Theorem below). By expressing the upper boundary condition  
 192 in this way, we are assuming that mass transfer across the SWI is rate-limited by the

mixing of solute within the streambed and not by convective mixing across the concentration boundary layer above the streambed (Grant et al., 2018). Put another way, we are assuming that the Biot Number—which expresses the ratio of timescales for diffusive mixing in the streambed and convective mass transfer across the turbulent boundary layer above the streambed—is much greater than unity (Incropera et al., 2007).

One of two lower boundary conditions can be selected, depending on whether the sediment bed is assumed to be finite (equation (3d)) or semi-infinite (equation (3e)) in extent.

$$\left. \frac{\partial c_s}{\partial \bar{y}} \right|_{\bar{y}=\bar{d}_b} = 0 \quad (3d)$$

$$c_s(\bar{y} \rightarrow \infty, \bar{t}) = 0 \quad (3e)$$

Equation (3d) enforces a no-flux boundary condition at scaled depth  $\bar{d}_b = a d_b$  [-] where  $d_b$  [L] is the depth of the sediment bed (**Figure 1**). Equation (3e) implies that, very deep into the bed ( $\bar{y} \rightarrow \infty$ ), the interstitial concentration is maintained at its initial state.

## 2.2 Duhamel's Theorem and Green's Functions

We seek an explicit solution to the above system of equations that is valid for any time-varying solute concentration in the overlying water column, any piecewise continuous diffusivity profile, and either a finite or semi-infinite streambed. To this end we invoke Duhamel's Theorem, an analytical approach for solving the diffusion equation in cases where one boundary is a piece-wise continuous function of time (Perez Guerrero et al., 2013). Proofs of this theorem typically assume that the diffusion coefficient is constant. However, as demonstrated in the Supplemental Information (**Text S1**), the theorem also applies in cases where the diffusion coefficient varies solely as a function of depth.

215 For the version of Duhamel's Theorem adopted here, three conditions must be  
 216 met (Myers, 1971): (1) the system must have a zero initial state; (2) the differential  
 217 equation and boundary conditions must be homogeneous with the exception of a single  
 218 time-dependent boundary condition or source/sink term in the differential equation; and  
 219 (3) the single nonhomogeneous term should be initially equal to zero. By design, our  
 220 system meets all three requirements. Accordingly, Duhamel's Theorem allows us to  
 221 express the interstitial solute concentration in the sediment bed as a convolution of the  
 222 time-derivative of the water column concentration  $c_w(\bar{t})$  and a so-called *auxiliary*  
 223 *function*  $c_s^A(\bar{y}, \bar{t})$  where  $v$  is a dummy integration variable (Meyers, 1971):

$$224 \quad c_s(\bar{y}, \bar{t}) = \int_0^{\bar{t}} c_s^A(\bar{y}, \bar{t} - v) \frac{d}{dv} [c_w(v) H(v)] dv \quad (4a)$$

225 The auxiliary function is, by definition, a solution to the same system of equations  
 226 described above for the interstitial solute concentration (equations (2b) - (3e)) but with  
 227 the nonhomogeneous term replaced by unity (compare equations (3b) and (4d)):

$$228 \quad \frac{\partial c_s^A}{\partial \bar{t}} = \frac{\partial}{\partial \bar{y}} \left( f(\bar{y}) \frac{\partial c_s^A}{\partial \bar{y}} \right) \quad (4b)$$

$$229 \quad c_s^A(\bar{y}, \bar{t} = 0) = 0, \bar{y} \geq 0 \quad (4c)$$

$$230 \quad c_s^A(\bar{y} = 0, \bar{t}) = H(\bar{t}) \quad (4d)$$

231 The auxiliary function's lower boundary condition depends on whether the sediment bed  
 232 is finite or semi-infinite in extent:

$$233 \quad \left. \frac{\partial c_s^A}{\partial \bar{y}} \right|_{\bar{y}=\bar{d}_b, \bar{t}} = 0 \quad (4e)$$

$$234 \quad c_s^A(\bar{y} \rightarrow \infty, \bar{t}) = 0 \quad (4f)$$

235 Duhamel's Theorem (equation (4a)) can also be expressed as a convolution of the  
 236 dimensionless water column concentration,  $c_w(\bar{t})$ , and a so-called Green's function,  
 237  $G(\bar{y}, \bar{t})$  [ $T^{-1}$ ], scaled here by the mixing timescale introduced earlier,  $\bar{G}(\bar{y}, \bar{t}) = t_r G(\bar{y}, \bar{t})$  (see  
 238 **Text S2** for details):

$$239 \quad c_s(\bar{y}, \bar{t}) = \int_0^{\bar{t}} \bar{G}(\bar{y}, \nu) c_w(\bar{t} - \nu) d\nu \quad (5a)$$

$$240 \quad \bar{G}(\bar{y}, \bar{t}) = \frac{\partial c_s^A}{\partial \bar{t}} \quad (5b)$$

241 According to equation (5a), solute concentration in the interstitial fluid of the sediment  
 242 bed at any depth and time,  $c_s(\bar{y}, \bar{t})$ , depends on the entire prior history of solute  
 243 concentration in the water column,  $c_w(\bar{t})$ , filtered through the Green's function,  $\bar{G}(\bar{y}, \bar{t})$ .  
 244 The Green's function, in turn, is a fundamental solution to the diffusion equation  
 245 (equation (2b)) that characterizes the response of solute concentration in the interstitial  
 246 fluids of the streambed to an impulsive injection of mass at the SWI at  $\bar{t} = 0$ ,  $c_w(\bar{t}) = \delta(\bar{t})$ ,  
 247 where  $\delta(\bar{t})$  [-] is the Dirac Delta function. Because the Green's function is calculated  
 248 from the auxiliary function (equation (5b)) its functional form will depend on the vertical  
 249 structure of the diffusivity profile,  $f(\bar{y})$ , and the lower boundary condition (either  
 250 equation (4e) or (4f)). Five Green's functions, corresponding to different combinations of  
 251 the diffusivity profile and lower-boundary condition, are derived in **Section 2.4**.

## 252 **2.3 Two-Way Coupling Across the SWI in a Closed System**

253 In a typical application of Duhamel's Theorem, the time-dependence of the  
 254 nonhomogeneous boundary condition is stipulated in advance. In our case, however, the

nonhomogeneous boundary condition (i.e., the water column concentration, equation (3b)) depends on mass flux across the SWI through a mass balance over the water column. For a closed system with a well-mixed water column, such as the stirred tank in **Figure 1b**, the water column mass balance takes on the following form:

$$A_b h_w \frac{dC_w}{dt} = A_b \theta D_{\text{eff},0} \left. \frac{\partial C_s}{\partial y} \right|_{y=0,t} \quad (6a)$$

In this equation, the change of solute mass in the water column (left hand side) equals the rate of mass transfer across the SWI by turbulent mixing (right hand side). New variables appearing here include the interfacial area,  $A_b$  [ $\text{L}^2$ ], of the streambed and the height of the water column,  $h_w$  [ $\text{L}$ ]. Streambed porosity,  $\theta$ , is included on the right-hand side of the equation to account for the streambed's porosity; specifically, the abrupt change in area over which solute mass transport occurs above and below the SWI (Grant et al., 2012). Expressing equation (6a) using the dimensionless variables introduced earlier, we obtain equation (6b) where the dimensionless variable,  $\bar{h}_w$  (equation (6c)), is a scaled form of the water column depth.

$$\frac{dc_w}{d\bar{t}} = \frac{1}{\bar{h}_w} \left. \frac{\partial c_s}{\partial \bar{y}} \right|_{\bar{y}=0,\bar{t}} \quad (6b)$$

$$\bar{h}_w = \frac{a h_w}{\theta} \quad (6c)$$

Two-way coupling across the SWI manifests mathematically as a dependence of the water column concentration (left hand side, equation (6b)) on the streambed's interstitial solute concentration (right hand side, equation (6b)) and, simultaneously, the dependence of the streambed's interstitial solute concentration (left hand side, equation (5a)) on the solute concentration in the water column (through its convolution with the

276 Green's function, right hand side equation (5a)). This two-way coupling can be solved  
 277 exactly by manipulating the water and sediment mass balance equations in the Laplace  
 278 Domain. As demonstrated in Supplemental Information (**Text S3**), the result is a set of  
 279 fully coupled solutions for solute concentration in the water and sediment columns of a  
 280 closed system with two-way coupling across the SWI "turned on":

$$281 \quad c_w(\bar{t}) = (c_{w0} - c_{s0}) \mathcal{L}^{-1} \left[ \frac{1/\bar{s}}{1 - \frac{1}{\bar{s}h_w} \left( \partial \tilde{G} / \partial \bar{y} \right)_{\bar{y}=0, \bar{s}}} \right] + c_{s0} \quad (7a)$$

$$282 \quad c_s(\bar{y}, \bar{t}) = (c_{w0} - c_{s0}) \mathcal{L}^{-1} \left[ \frac{\tilde{G}(\bar{y}, \bar{s})/\bar{s}}{1 - \frac{1}{\bar{s}h_w} \left( \partial \tilde{G} / \partial \bar{y} \right)_{\bar{y}=0, \bar{s}}} \right] + c_{s0} \quad (7b)$$

283 In these solutions, the symbol  $\mathcal{L}^{-1}[\cdot]$  represents the inverse Laplace Transform,  $\bar{s} = st_T$  [-]  
 284 is a dimensionless form of the Laplace Transform variable  $s$  [ $T^{-1}$ ], and  $\tilde{G}$  is the Laplace  
 285 transform of the dimensionless Green's function which, in turn, depends on the  
 286 diffusivity depth profile  $f(\bar{y})$  and bottom boundary condition (through the solution to the  
 287 auxiliary function (equation (5b)). A corresponding set of solutions can also be derived  
 288 for when two-way coupling across the SWI is "turned off"; i.e., the diffusion equation's  
 289 upper boundary condition is held at its initial state,  $c_s(\bar{y}=0, \bar{t}) = c_{w0}$ :

$$290 \quad c_w(\bar{t}) = (c_{w0} - c_{s0}) \mathcal{L}^{-1} \left[ \frac{1}{\bar{s}} \left( \frac{1}{\bar{s}h_w} \frac{\partial \tilde{G}}{\partial \bar{y}} \Big|_{\bar{y}=0, \bar{s}} + 1 \right) \right] + c_{s0} \quad (7c)$$

$$291 \quad c_s(\bar{y}, \bar{t}) = (c_{w0} - c_{s0}) \mathcal{L}^{-1} \left[ \frac{\tilde{G}(\bar{y}, \bar{s})}{\bar{s}} \right] + c_{s0} \quad (7d)$$

292 [Figure 2 goes about here]

Depending on the choice of Green's function, the inverse Laplace transforms appearing in equations (7a) - (7d) were either determined analytically, or numerically evaluated using Gaussian Quadrature implemented in the Mathematica package authored by U. Graf (Graf, 2004).

## 2.4 Laplace Domain Solutions for the Green's Function

In **Table 1** we present five Laplace domain solutions for the Green's function given four different choices of the diffusivity depth profile and two different choices of the bottom boundary condition (finite or semi-infinite sediment bed) (derivations in Supplemental Information, **Text S4**). Together with equations (7a) through (7d), these five Green's functions provide a total of 20 different solution combinations for solute concentration in the water and sediment column of a closed system with two-way coupling across the SWI turned on (equations (7a) and (7b)) or off (equations (7c) and (7d)). The four diffusivity depth profiles we evaluated include (**Figure 2**): (1) constant (*C Profile*, equation (8a)); (2) exponentially declining (*E Profile*, equation (8b)); (3) exponentially declining to a tortuosity-modified molecular diffusion coefficient (*E2M Profile*, equation (8c)); and (4) constant to exponentially declining diffusivity (*C2E Profile*, equation (8d)).

$$f_c(\bar{y})=1, \quad \bar{y}=a_c y \quad (8a)$$

$$f_E(\bar{y})=e^{-\bar{y}}, \quad \bar{y}=a_E y \quad (8b)$$

$$f_{E2M}(\bar{y})=\begin{cases} e^{-\bar{y}}, & 0 \leq \bar{y} \leq \bar{\ell}_m \\ \bar{D}=D'_m/D_{\text{eff},0}^{E2M}, & \bar{y} > \bar{\ell}_m \end{cases}, \quad \bar{\ell}_m = -\ln \bar{D}, \quad 0 < \bar{D} < 1, \quad \bar{y}=a_{E2M} y \quad (8c)$$

$$f_{C2E}(\bar{y})=\begin{cases} 1, & 0 \leq \bar{y} \leq \bar{\ell}_t \\ e^{-(\bar{y}-\bar{\ell}_t)}, & \bar{y} > \bar{\ell}_t \end{cases}, \quad \bar{\ell}_t = a_{C2E} \ell_t, \quad \bar{y}=a_{C2E} y \quad (8d)$$



313 Most laboratory (Marion and Zaramella, 2015; O'Connor and Harvey, 2008; Grant et al.,  
 314 2012) and field (Wörman, 2000) studies of diffusive mixing across the SWI adopt the C  
 315 Profile. However, several studies (He et al., 2019; Roche et. al, 2019; Chandler et al.,  
 316 2016; Nagaoka and Ohgaki, 1990) have shown that turbulent mixing in the sediment bed  
 317 declines exponentially with depth, and a recent numerical modeling study concluded that  
 318 the E Profile is consistent with experimental breakthrough curves measured in the  
 319 laboratory and field (Bottacin-Busolin, 2019). The E2M profile is a natural extension of  
 320 the E Profile that accounts for the fact that the tortuosity-modified molecular diffusion  
 321 coefficient  $D'_m$  [ $L^2 T^{-1}$ ] imposes a lower-bound on the effective diffusivity. E2M's  
 322 mathematical representation (equation (8c)) includes two new variables: the  
 323 dimensionless depth at which the diffusivity profile transitions from exponentially  
 324 declining to the tortuosity-modified molecular diffusion coefficient,  $\bar{\ell}_m = a_{E2M} \ell_m$  [-] where  
 325  $\ell_m$  [L] is the transition depth and  $\bar{D} = D'_m / D_{eff,0}^{E2M}$  [-] is a dimensionless form of the  
 326 tortuosity-modified molecular diffusion coefficient. These two parameters are  
 327 mathematically related as follows:  $\bar{\ell}_m = -\ln \bar{D}$ . Finally, the C2E Profile captures enhanced  
 328 mixing at the top of the streambed by extending the surficial effective diffusivity  $D_{eff,0}^{C2E}$  to  
 329 a depth  $y = \ell_t$  [L] below the SWI. For depths greater than the constant mixing thickness,  
 330  $y > \ell_t$ , the diffusivity profile declines exponentially. The dimensionless form of the  
 331 enhanced mixing thickness  $\ell_t$  is defined in the usual way,  $\bar{\ell}_t = a_{C2E} \ell_t$ .

332 [Table 1 goes about here]

### 333 3 Example of the Theory's Application to Mixing Across the SWI in a Stirred Tank

334 As a tangible example of the theory presented in **Section 2**, consider an experiment  
 335 (along the lines examined in detail later, see **Section 4**) in which a mass  $M$  of a  
 336 conservative solute is added to the interstitial fluids of a sediment bed in an otherwise  
 337 solute-free stirred tank. Adopting the notation indicated in **Figure 1b**, the initial  
 338 interstitial solute concentration in the sediment bed will be:  $C_{s0} = M / (d_b A_b \theta)$ . At time  $t = 0$   
 339 the impeller is turned on causing solute concentration in the overlying water column to  
 340 increase as solute turbulently mixes out of the bed. If the experiment runs long enough,  
 341 the solute concentration in the water column and interstitial fluids of the sediment bed  
 342 will approach a final (well-mixed) equilibrium concentration,  $C_{eq}$  [ $M L^{-3}$ ]:

$$343 \quad \frac{C_{eq}}{C_{s0}} = \frac{\bar{d}_b}{1 + \bar{d}_b}, \quad \bar{d}_b = d_b \theta / h_w \quad (9a)$$

344 How does solute concentration in the overlying water column and interstitial fluids of the  
 345 sediment bed evolve from their initial state (where all solute mass is located in the  
 346 sediment bed) to the final (well-mixed) state given by equation (9a)? Within the context  
 347 of our modeling framework, the answer depends on the depth-dependence of the  
 348 diffusivity profile, whether two-way coupling across the SWI is turned “on” or “off”, and  
 349 whether the sediment bed is assumed to be semi-infinite or finite in extent.

350 As an illustration we adopt the C Profile and derive from the theoretical  
 351 framework in **Section 2** the following three explicit solutions for the evolution of solute  
 352 concentration in the water column (see **Text S5** for details): (1) a “null model” which  
 353 assumes two-way coupling across the SWI is turned off and the sediment bed is infinitely  
 354 deep (equation (9b)); (2) an “infinite bed” model that assumes two-way coupling is  
 355 turned on and the sediment bed is infinitely deep (equation (9c)); and (3) a “finite bed”

356 model that accounts for both two-way coupling across the SWI and the finite depth of the  
 357 sediment bed (equation (9d)).

$$358 \quad \frac{C_{w,null}^c(\bar{t}_c)}{C_{s0}} = 2\sqrt{\frac{\bar{t}_c}{\pi}}, \quad \bar{t}_c = \theta^2 D_{eff,0}^c t / h_w^2 \quad (9b)$$

$$359 \quad \frac{C_{w,\infty}^c(\bar{t}_c)}{C_{s0}} = 1 - e^{-\bar{t}_c} \operatorname{erfc}\left(\sqrt{\bar{t}_c}\right) \quad (9c)$$

$$360 \quad \frac{C_{w,finite}^c(\bar{t}_c)}{C_{s0}} = 1 - \mathcal{L}^{-1} \left[ \frac{1/\bar{s}}{1 + \tanh(\bar{d}_b \sqrt{\bar{s}}) / \sqrt{\bar{s}}} \right] \quad (9d)$$

361 The corresponding set of solutions for solute concentration in the interstitial fluids of the  
 362 sediment bed are as follows:

$$363 \quad \frac{C_{s,null}^c(\bar{y}_c, \bar{t}_c)}{C_{s0}} = \operatorname{erfc}\left(\frac{\bar{y}_c}{\sqrt{4\bar{t}_c}}\right), \quad \bar{y}_c = \theta y / h_w \quad (10a)$$

$$364 \quad \frac{C_{s,\infty}^c(\bar{y}_c, \bar{t}_c)}{C_{s0}} = 1 - e^{-\bar{t}_c + \bar{y}_c} \operatorname{erfc}\left(\frac{2\bar{t}_c + \bar{y}_c}{\sqrt{4\bar{t}_c}}\right) \quad (10b)$$

$$365 \quad \frac{C_{s,finite}^c(\bar{y}_c, \bar{t}_c)}{C_{s0}} = 1 - \mathcal{L}^{-1} \left[ \frac{\cosh((\bar{d}_b - \bar{y}_c)\sqrt{\bar{s}})}{\bar{s} \cosh(\bar{d}_b \sqrt{\bar{s}}) + \sqrt{\bar{s}} \sinh(\bar{d}_b \sqrt{\bar{s}})} \right] \quad (10c)$$

366 The superscript “C” indicates that these solutions are specific to the C Profile.

367 The null model predicts that solute concentration in the water column increases  
 368 without bound and in proportion to the square root of time (i.e., solute concentration in  
 369 the water column increases linearly when plotted against  $\sqrt{\bar{t}_c}$ , thick black dashed line,  
 370 **Figure 3a**). The infinite bed model rises with the null model initially (i.e., until around  
 371  $\sqrt{\bar{t}_c} \approx 0.25$ ) but then slows as two-way coupling reduces the rate of mass transfer across  
 372 the SWI (thin black solid curve in **Figure 3a**). The finite bed model exhibits three phases

(colored dashed lines in the figure): tracking the null solution early on, transitioning to the infinite bed solution at intermediate times, and stabilizing at a final equilibrium concentration at long times (thin horizontal colored lines).

[Figure 3 goes about here]

Similar patterns are evident for model-predicted solute concentration in the interstitial fluids of the streambed (**Figure 3b**). For these simulations we focused on the temporal evolution of interstitial solute concentration in the shallow portion of the bed,  $\bar{y}_c = 0.05$ ; this particular depth was chosen so that we would not violate the inequality requirement,  $\bar{y}_c \leq \bar{d}_b$ , for the smallest dimensionless bed depth of  $\bar{d}_b = 0.1$  (light blue horizontal lines in **Figure 3**). The null model (thick black dashed curve) predicts a steep drop in solute concentration initially (as solute in the upper portion of the streambed mixes into a solute-free water column) followed by a gradual decline with time (as solute from deeper in the bed mixes upward). The infinite bed solution (thin black curve) declines with the null model initially, but then rebounds as two-way coupling slows mass transfer across the SWI. Indeed, this rebound closely approximates the rise in water column concentration predicted by the infinite bed model (compare thin black curves in **Figures 3a** and **3b**) implying that, when two-way feedback is turned on, solute concentrations in the interstitial fluids near the top of the sediment bed are similar to the solute concentration in the overlying water column. Interstitial solute concentrations predicted by the finite bed model (colored dashed curves in **Figure 3b**) exhibit the same three phases noted above for the water column; namely, they track the null model at early times, follow the infinite bed model at intermediate times, and stabilize at an equilibrium concentration at long times. For the model-data comparisons described in the next section

we employ a set of infinite bed models derived for each of the four diffusivity profiles (C, E, E2M, and C2E) and restrict the experimental time window to well before the onset of equilibrium conditions.

## **4 Applying the Theory to Previously Published Measurements**

### **4.1 Chandler et al.'s Experiments**

Next we turn to an extensive set of previously published measurements of turbulent mixing of a conservative tracer (Rhodamine WT) across the SWI in a stirred tank with a flat sediment bed, along the lines of the experimental set-up illustrated in **Figure 1b** (Chandler et al., 2016; Chandler, 2012). Chandler et al.'s study is notable for several reasons. First, it is one of the few where tracer concentrations were simultaneously measured in the water and sediment columns allowing us to directly compare mixing parameters estimated from data collected exclusively above or below the SWI. Second, the twenty (out of twenty-six total) Chandler et al. experiments included in this study cover a range of bed shear velocities ( $u_* = 0.01$  to  $0.04 \text{ m s}^{-1}$ ), mean grain diameters ( $d_g = 0.15$  to  $5.00 \text{ mm}$ ), and sediment permeabilities ( $K = 0.18$  to  $223 \text{ m}^2$ ). Finally, by conceptually dividing the sediment bed into a series of layers and fitting the diffusion equation (with a constant diffusivity profile) to each layer separately, Chandler et al. concluded that the diffusivity declines exponentially with depth. Nagaoka and Ohgaki (1990), who pioneered this approach, reached a similar conclusion. Thus, there is already a strong indication that the diffusivity in their system declines with depth. One caveat is that Chandler et al.'s range of shear velocities is on the low side for some streams; e.g., the 72 headwater streams included in the U.S. Lotic Intersite Nitrogen eXperiment (LINX II) had shear velocities ranging from  $0.02$  to  $0.48 \text{ m s}^{-1}$  and only seven streams had shear

419 velocities in the range interrogated by Chandler et al. (Hall et al., 2009). However, when  
420 the shear velocity and permeability for each of Chandler et al.’s experiments are  
421 combined to calculate a Permeability Reynolds Number, this master variable spans both  
422 dispersive and turbulent diffusive mixing regimes (see **Section 5.1**).

423         Details of Chandler et al.’s experiments and the approach we used for parameter  
424 estimation and model performance evaluation are briefly described here (see **Text S6** for  
425 details). In brief, the sediment column, which had a depth of  $d_b = 0.2$  m and a porosity of  
426  $\theta = 0.38$  to  $0.39$ , consisted of randomly packed single-sized spherical soda glass spheres.  
427 In all experiments, the initial state was a Rhodamine WT saturated sediment bed  
428 (concentration of  $C_{s0} = 100$  ppb) and a Rhodamine-free water column ( $C_{w0} = 0$  ppb)  
429 although the actual concentrations varied somewhat by experiment (**Table S1**).  
430 Experiments were initiated when the impeller motor was turned on, whereupon tracer  
431 concentrations were fluorometrically monitored in the water column and at 5 depths in  
432 the sediment column ( $x = 0.015, 0.049, 0.083, 0.117, \text{ and } 0.151$  m below the SWI) at a  
433 frequency of  $0.1$  Hz over a period of hours to days. Diffusivity profile parameters were  
434 inferred by minimizing the root mean squared error (RMSE) calculated from  
435 observations and profile-specific infinite bed model predictions using the non-linear least  
436 squares algorithms implemented within the “NonlinearModelFit” command in the  
437 Mathematica computing package (v. 11.20, Wolfram Research, Inc.). Several model  
438 performance metrics were also generated, including RMSE, coefficient of determination  
439 ( $R^2$  value), and the corrected Akaike Information Criterion (AICc). The latter was used to  
440 rank the performance of the four diffusivity profile models, accounting for the trade-off  
441 between model fit and model complexity. The top-ranked (most parsimonious) model has

the smallest AICc value (Aho et al., 2014). Because model predictions were based on “infinite bed models” specific to each profile (see **Section 3**), we restricted the experimental window to periods when the bottom interstitial Rhodamine WT concentration, as measured by the deepest probe, was within 90% of its initial value (see  $t_{\text{final}}$  values in **Table S1**). By this approach, diffusivity profile parameters were estimated for all four diffusivity profiles (C, E, E2M, and C2E) and for 20 of Chandler et al.’s experiments (inferred parameter values, estimated errors, and model performance metrics are summarized in **Tables S2-S7**); six experiments were excluded due to missing data or other issues.

## 4.2 Experimental Evaluation of the C Profile

Significant bias is evident when tracer concentrations predicted by the C Profile’s infinite bed model are compared to Chandler et al.’s experimental data (**Figure 4a**). In this figure we have plotted Chandler et al.’s Rhodamine WT measurements against the square root of dimensionless time,  $\sqrt{t/t_c} = \sqrt{t/t_c}$ , where the time-constant,  $t_c = h_w^2 / (\theta^2 D_{\text{eff},0}^c)$ , varies from experiment-to-experiment depending on the inferred value of the C Profile’s effective diffusivity (summarized in **Table S2**); the other two parameters,  $h_w$  and  $\theta$ , varied little across Chandler et al.’s twenty experiments. When plotted in this way Chandler et al.’s data can be compared directly to a single model-predicted curve for the time evolution of Rhodamine WT concentration in the water column (solid black curve, equation (9c), top panel in **Figure 4a**) and at two depths (15 or 151 mm below the SWI) in the sediment column (solid black curves, equation (10b), bottom panel in **Figure 4a**). For clarity, Rhodamine WT measurements at the three intermediate depths (4.9, 8.3, and 117 cm

below the SWI) were not included this figure, although these data were included in the model optimization studies described in **Section 5.1**.

[Figure 4 goes about here]

By virtue of the way Chandler et al.'s experiments were conducted, the Rhodamine WT concentration in the water column is proportional to the cumulative Rhodamine WT mass transferred from the sediment bed to the water column over time. The results plotted in the top panel of **Figure 4a** therefore imply that the C Profile model under- and over-estimates mass transfer out of the sediment bed at short and long times, respectively. A comparison of model-predicted and measured Rhodamine WT concentrations in the sediment bed reveals the problem (bottom panel, **Figure 4a**): the C Profile model under-estimates mixing in the surficial portion of the bed at early times (i.e., predicted concentrations exceed measured concentrations at 15 mm below the SWI) and over-estimates mixing deeper in the bed at later times (i.e., predicted concentrations are less than measured concentrations at 151 mm below the SWI).

### 4.3 Experimental Evaluation of the E Profile

The model bias described above is reduced substantially when the effective diffusivity decays exponentially with depth (**Figure 4b**). The E Profile's infinite bed model was constructed by substituting its Green's function (equation (T3)) into the water and sediment mass balance solutions (equations (7a) and (7b)) for a closed system with two-way coupling across the SWI "turned on" (the superscript "E" indicates that these solutions are specific to the E Profile):

$$C_{w,\infty}^E(\bar{t}_E) = (C_{w0} - C_{s0}) \mathcal{L}^{-1} \left[ \frac{\bar{h}_w K_1(2\sqrt{s})}{\bar{s} \bar{h}_w K_1(2\sqrt{s}) + \sqrt{s} K_0(2\sqrt{s})} \right] + C_{s0} \quad (11a)$$



$$C_{s,\infty}^E(\bar{y}, \bar{t}_E) = (C_{w0} - C_{s0}) \mathcal{L}^{-1} \left[ \frac{\bar{h}_w \sqrt{e^{\bar{y}}} K_1(2\sqrt{e^{\bar{y}} \bar{s}})}{\bar{s} \bar{h}_w K_1(2\sqrt{\bar{s}}) + \sqrt{\bar{s}} K_0(2\sqrt{\bar{s}})} \right] + C_{s0} \quad (11b)$$

$$\bar{t}_E = t/t_E, \quad t_E = 1/(a_E^2 D_{\text{eff},0}^E), \quad \bar{h}_w = a_E h_w / \theta, \quad \bar{y} = a_E y \quad (11c)$$

Because the E Profile model has two unknown parameters ( $D_{\text{eff},0}^E$  and  $a_E$ , as opposed to the C Profile's one unknown parameter,  $D_{\text{eff},0}^C$ ) there is no longer a single “master” curve against which all of Chandler et al.'s water column data, for example, can be compared to (as there was for the C Profile in the top panel of **Figure 4a**). Instead, for the E Profile (and the C2E Profile described below) model-data comparisons must be conducted on an experiment-by-experiment basis. In **Figure 4b** we therefore focus our model-data comparison on a typical experiment (red symbols in the figure, experiment ID #20110613). To generate model predictions for this experiment we: (1) fit equation (11a) to the experiment's water column data yielding estimates for the inverse depth-scale and effective diffusivity ( $a_E = 5.0 \pm 1.2 \text{ m}^{-1}$  and  $D_{\text{eff},0}^E = (5.6 \pm 0.5) \times 10^{-6} \text{ m}^2 \text{ s}^{-1}$ ); and (2) plotted the model against the same abscissa used in **Figure 4a** after applying the following dimensionless time transformation,  $\bar{t}_E = (\bar{h}_w^2 D_{\text{eff},0}^E / D_{\text{eff},0}^C) \bar{t}_C$ , and substituting experiment-specific values for the dimensionless water depth ( $\bar{h}_w = a_E h_w / \theta = 33 \pm 14$ ) and the C Profile's effective diffusivity ( $D_{\text{eff},0}^C = (3.4 \pm 0.2) \times 10^{-7} \text{ m}^2 \text{ s}^{-1}$ ). Equation (11a) closely reproduces the experiment's water column concentrations of Rhodamine WT (compare solid curve and red points in **Figure 4b**, top panel), although some model bias is evident for  $\sqrt{\bar{t}_C} < 0.05$ . The E Profile's infinite bed model (equation (11b)) also captures the fast and slow mixing of Rhodamine WT out of the shallow and deep portions of the sediment bed (compare

black curves and red points, lower panel, **Figure 4b**). The last result is notable given that the parameter values used to generate these predictions were inferred from the water column data; i.e., by fitting the E Profile's infinite bed model to measurements of Rhodamine WT in the water column we can predict the evolution of Rhodamine WT in the interstitial fluids of the sediment bed with reasonable fidelity.

#### 4.4 Experimental Evaluation of the E2M Profile

Setting the tortuosity-modified molecular diffusion coefficient,  $D'_m$ , as a lower bound on the effective diffusivity does not improve the E Profile's model performance. We arrived at this conclusion by estimating a tortuosity-modified diffusion coefficient for Chandler et al.'s experiments ( $D'_m = 1.31 \times 10^{-10} \text{ m}^2\text{s}^{-1}$ ) from the ratio of the molecular diffusion coefficient for Rhodamine in water at 21°C ( $D_m = 2.9 \times 10^{-10} \text{ m}^2\text{s}^{-1}$ ) (Chandler 2012) and the tortuosity of the sediment bed ( $\tau = 2.22$ ), estimated by substituting the measured bed porosity ( $\theta = 0.39$ ) into an empirical formula proposed by Iversen and Jorgensen (1992) for sandy beds:  $\tau = 1 + 2(1 - \theta)$ . Values of the inverse depth-scale and effective diffusivity,  $a_{\text{E2M}}$  and  $D_{\text{eff},0}^{\text{E2M}}$ , were then inferred by fitting the E2M Profile's infinite bed model (constructed by substituting equations (T5a,b,c) in **Table 1** into equations (7a) and (7b)) to Chandler et al.'s water column Rhodamine WT measurements. Repeating this process for all twenty of Chandler et al.'s experiments reveals that the E2M's inverse depth-scales and effective diffusivities are nearly identical to those inferred from the E Profile model (**Figure S1**, also compare **Tables S4** and **S6**), implying that these two profiles are functionally equivalent. This result can be rationalized by noting that the timescale for mixing through the sediment column by molecular diffusion ( $t_m = \ell_m^2 / D'_m \approx 11$  months,

where the mixing depth is  $\ell_m = -\ln(D'_m/D_{\text{eff},0}^{\text{E2M}})/a_{\text{E2M}}$ , see equation (8c) and discussion thereof) is much longer than the time period Chandler et al. conducted their experiments (days to weeks). While the E2M Profile did not add value (relative to the E Profile) to our analysis of Chandler et al.'s experiments, it might prove useful in settings with very low levels of turbulence and relatively impermeable sediments (i.e., very small values of the Permeability Reynolds Number, see **Section 5**).

#### 4.5 Experimental Evaluation of the C2E Profile

The C2E Profile's infinite bed model provides a near perfect representation of Chandler et al.'s water column data (**Figure 4c**). The C2E's infinite bed model was constructed by substituting its Green's function (equations (T6a) - (T6c) in **Table 1**) into equations (7a) and (7b). We then followed the same two-step procedure outlined in **Section 3.3**, by (1) fitting the model to Rhodamine concentrations measured in the water column during Exp ID# 20110613 ( $D_{\text{eff},0}^{\text{C2E}} = (1.5 \pm 0.07) \times 10^{-6} \text{ m}^2 \text{ s}^{-1}$ ,  $\ell_t = 0.04 \pm 0.002 \text{ m}$ ,  $a_{\text{C2E}} = 66 \pm 3.1 \text{ m}^{-1}$ ); and (2) plotting the model against the same abscissa used in **Figures 4a,b** after applying the following dimensionless time transformation,  $\bar{t}_{\text{C2E}} = (\bar{h}_w^2 D_{\text{eff},0}^{\text{C2E}} / D_{\text{eff},0}^{\text{C}}) \bar{t}_{\text{C}}$ , and substituting experiment-specific values for the dimensionless water depth ( $\bar{h}_w = a_{\text{C2E}} h_w / \theta = 44 \pm 19$ ) and the C Profile's effective diffusivity ( $D_{\text{eff},0}^{\text{C}} = (3.4 \pm 0.2) \times 10^{-7} \text{ m}^2 \text{ s}^{-1}$ ). As noted above, the C2E Profile's infinite bed model provides a near perfect match to Rhodamine WT concentrations measured in the water column (compare solid curve and red points in **Figure 4c**). However, when these same parameter values are used to estimate Rhodamine WT concentration in the interstitial fluids of the bed, there is some loss of model performance compared to the E Profile; i.e., the C2E model systematically

underestimates mixing in the streambed, especially for Rhodamine measurements at 15 mm below the SWI (compare lower panels in **Figures 4b** and **4c**).

## **5. Discussion**

Here we expand our focus to include all twenty of Chandler et al.'s experiments with the goal of answering the three questions raised in **Section 1**.

### **5.1 How is the Effective Diffusivity Structured Vertically?**

From refractive index matched particle image velocimetry (RIM-PIV) studies of turbulent motions across the SWI of a permeable streambed, Voermans et al. (2017) concluded that the flux of mass and momentum is dominated by different transport mechanisms (molecular diffusion, dispersion, turbulent diffusion) depending on the magnitude of the Permeability Reynolds Number,  $Re_K = u_* \sqrt{K} / \nu$ , a dimensionless arrangement of the shear velocity  $u_*$  [ $L T^{-1}$ ], sediment bed permeability  $K$  [ $L^2$ ], and kinematic viscosity of water  $\nu$  [ $L^2 T^{-1}$ ]. Specifically, molecular diffusion dominates mixing across the SWI for small values of this dimensionless number ( $Re_K < 0.01$ ), dispersion for intermediate values ( $0.01 < Re_K < 2$ ), and turbulent diffusion for large values ( $Re_K > 2$ ).

Chandler et al.'s dataset allows us to quantitatively compare the performance of the three diffusivity profiles (C, E, and C2E) over a Permeability Reynolds Number range (0.2 to 4.34) that spans dispersive and turbulent diffusive mixing regimes (note that the E2M profile is not included in this list because, as noted in **Section 4.4**, this profile is functionally equivalent to the E Profile in the context of Chandler et al.'s dataset). Based on our earlier analysis of Chandler et al.'s Exp ID 20110613 we concluded that the E Profile represents a substantial improvement over the C Profile (**Section 4.3**). This

conclusion extends to the rest of Chandler et al.'s experiments as well (compare blue and red circles in **Figure 5**, inferred parameter values summarized in **Tables S2 – S5**). In all cases, the E Profile's infinite bed model captures a larger fraction of data variance ( $R^2 > 99.5\%$ , **Figure 5a**) and has substantially smaller RMSE values (**Figures 5b**). The E Profile's AICc is also  $>10$  units lower than the C Profile's AICc (**Figure 5c**) implying that, despite its increased complexity (i.e., the C and E Profiles have one and two unknown parameters, respectively) the E Profile is the more parsimonious model (Weijss and Ruddell, 2020; Aho et al., 2014).

[Figure 5 goes about here]

The C2E Profile's infinite bed model also performs quite well, but is a problem with the inferred values of its inverse depth-scale. Compared to the E Profile, the C2E Profile's infinite bed model has consistently lower RMSE and AICC values (compare crosses and red circles in **Figures 5b** and **5c**) and a slightly improved coefficient of determination ( $R^2 > 99.8\%$ , **Figure 5a**). However, these improvements come at the cost of a new parameter (the C2E's inverse depth scale,  $a_{\text{C2E}}$ ) whose inferred values are poorly constrained (coefficient of variation in excess of 40 for some experiments, see **Table S7**), highly variable (varying over 1000-fold from experiment-to-experiment, see distribution of values represented by the violin plot in **Figure 6a**) and, in some cases, not physically meaningful (e.g., the implied depth over which the effective diffusivity decays for  $y > \ell_t$  is  $1/a_{\text{C2E}} \approx 20$  microns for the largest value of  $a_{\text{C2E}}$  indicated in **Figure 6a**). Inferred values of the C2E Profile's other two parameters (effective diffusivity,  $D_{\text{eff},0}^{\text{C2E}}$ , and depth of constant mixing,  $\ell_t$ ) are strongly correlated ( $R^2 = 0.93$  and  $0.76$ ) with the effective

diffusivity,  $D_{\text{eff},0}^E$ , and decay length-scale,  $1/a_E$ , inferred from the E Profile (**Figure 6b**, **6c**), implying that both models capture the same basic information about the vertical structure of the effective diffusivity; namely that the diffusivity is larger at the SWI and lower at depth. In summary, of the four profiles evaluated in this study and across the full range of Permeability Reynolds Numbers evaluated here (including dispersive and turbulent diffusive transport regimes, see horizontal axis of **Figure 5**) the E Profile is the most parsimonious descriptor of the effective diffusivity's vertical structure.

[Figure 6 goes about here]

## **5.2 Is the Flux-Gradient Diffusive Model an Accurate Representation of Turbulent Solute Transport Through the Streambed?**

We have already shown that the E Profile's infinite bed model captures a large percentage of the variance in Chandler et al.'s water column Rhodamine measurements ( $R^2 > 99.5\%$ , **Figure 5a**) but this assessment is based on the same datasets used for model calibration. A more rigorous test of the E Profile in particular, and the flux-gradient diffusive model more generally, can be stated as follows: *are the same E Profile parameter values obtained when the model is optimized with Rhodamine measured in the water column versus when the model is optimized with Rhodamine measured in the interstitial fluids of sediment bed?* Put another way, can the evolution of solute concentrations in the interstitial fluids of the sediment bed be inferred from the evolution of solute concentration in the water column, and vice versa?

The answer is a qualified “yes”. Across all 20 of Chandler et al.'s experiments, effective diffusivities  $D_{\text{eff},0}$  estimated from the water and sediment column data are strongly correlated over a 1000-fold change in the magnitude of this parameter (**Figure**

7a, Pearson's correlation coefficient,  $R=0.867$ ). Values of the inverse depth-scale inferred from the water and sediment column data are much less variable and not significantly correlated (**Figure 7b**), but their respective log-means ( $a=10^{1.61\pm0.18} \text{ m}^{-1}$  and  $a=10^{1.70\pm0.08}$ , respectively) are equal within error. The corresponding mean values ( $a_E=44.0\pm18.5 \text{ m}^{-1}$  and  $a=51.4\pm9.71 \text{ m}^{-1}$ , respectively, obtained by applying equations (S25a) and (S25b) in Supplemental Information) are also equal, within error, to the inverse depth-scale estimated by Chandler et al. in their original publication ( $a=55 \text{ m}^{-1}$ ) (Chandler et al., 2016). This inverse depth-scale corresponds to a “ $1/e$ -folding depth” (i.e., the depth at which the E Profile's effective diffusivity declines to  $1/e\approx0.37$  of its surficial value) of approximately 2 cm, which comports with field and laboratory estimates for the thickness of the benthic biolayer (2 to 5 cm below the SWI) (Tomasek et al., 2018; Knapp et al., 2017; Krause et al., 2017; Caruso et al., 2017; Trauth et al., 2014; Harvey et al., 2013; Kessler et al., 2013; Zarnetske et al., 2011; Battin et al., 2008; Dahm et al., 2002). Thus, turbulent mixing may play an outsized role in the biogeochemical processing of nutrients and other contaminants in streambed and coastal sediments.

[Figure 7 goes about here]

While effective diffusivities inferred from data collected above and below the SWI are strongly correlated, bias is evident for experiments with Permeability Reynolds Numbers greater than the approximate threshold for a fully turbulent SWI,  $Re_K > 10^{0.3} = 2$  (Voermans et al., 2017) (**Figure 7a**). One possible explanation focuses on how Chandler et al. measured Rhodamine concentrations in the interstitial fluids of the sediment bed. Chandler et al.'s in-bed measurements of Rhodamine WT are taken at a “point” (sensing volume ca.,  $0.23 \text{ cm}^3$ ) with fiber optic fluorometers, whereas our analytical framework

assumes that interstitial solute concentrations are horizontally averaged (**Section 2**). Indeed, Chandler (2010) describes how measured time series of concentration in the sediment were sensitive to fluorometer location and these differences appeared consistent over time; i.e. tracer appeared to mix out of the streambed faster on one side of the tank than on the other (ibid. pg. 173). These authors also document distinct and persistent patterns of mean flow velocity within the tank (ibid. pg. 118) which would lead to heterogeneous turbulence intensities and corresponding heterogeneous efflux across the SWI at high Permeability Reynolds Numbers. To the extent that Chandler et al.'s point measurements are not equal to horizontally averaged concentrations, the effective diffusivities inferred from these data will suffer accuracy problems. Indeed, Chandler (2010) noted an order of magnitude discrepancy in the timescale over which interstitial Rhodamine WT concentration declined on opposite sides of the stirred tank—a discrepancy that could induce order-of-magnitude inaccuracy in estimated sediment-side diffusivities (the scale of disagreement seen in **Figure 7a**).

An alternative explanation is that the flux-gradient diffusive model (equation (1a)) is an imperfect descriptor of turbulent mass transfer in the interstitial fluids of the sediment bed at high Permeability Reynolds Number—a conclusion supported by the systematic loss of model performance (i.e., higher RMSE and lower AICc and  $R^2$  values) observed at large values of this dimensionless number (i.e.,  $Re_k > 1$ , **Figure 5**). Indeed, the flux-gradient description of momentum transfer across other types of “porous” boundaries, such as vegetation canopies, can break down; i.e., significant momentum flux can occur in the absence of a velocity gradient, the analog of a concentration gradient for mass transfer (Ghisalberti and Nepf, 2002). However, such observations typically occur



in the dispersive transport regime ( $0.01 < \text{Re}_k < 2$ ) where our estimates of effective diffusivity above and below the SWI are in general concordance (**Figure 7a**). It should also be noted that, even for the highest Permeability Reynolds Numbers trialed here and so long as the diffusivity is allowed to decay exponentially with depth, the flux-gradient diffusive model still explains a large fraction of variance in water column measurements of Rhodamine WT ( $R^2 > 0.995$ ) (**Figure 5a**).

In summary, based on Chandler et al.'s laboratory measurements we can conclude the flux-gradient diffusive model is a reasonable representation of turbulent mass transfer in the interstitial fluids of the sediment bed, *provided that the vertical structure of the effective diffusivity is correctly specified (e.g., with the E Profile)*. However, a systematic loss of model performance is observed with increasing Permeability Reynolds Number in the turbulent diffusive regime ( $\text{Re}_k > 2$ ) possibly reflecting non-idealities associated with Chandler et al.'s experimental measurements, a more fundamental breakdown in the flux-gradient diffusive model, or some combination thereof. Further studies along these lines, particularly in field settings, are warranted.

### **5.3 Can Laboratory Measurements of Turbulent Mixing across the SWI be Extrapolated to Stream and Coastal Sediments?**

Translation of these results to the field requires “scaling relationships” from which the E Profile's two parameters—the effective diffusivity and inverse depth-scale—can be estimated. Over the years a number of researchers have reported that effective diffusivities (inferred by fitting the C Profile's null model to flume measurements of turbulent mixing across flat sediment beds) exhibit a quadratic dependence on the Permeability Reynolds Number,  $D_{\text{eff},0}^C \propto \text{Re}_k^2$  (Richardson and Parr, 1988; O'Connor and

Harvey, 2008; Grant et al., 2012; Voermans et al., 2018; Grant et al., 2018). As noted earlier, the Permeability Reynolds Number is calculated from the shear velocity,  $u_*$ , sediment bed permeability,  $K$ , and the kinematic viscosity of water,  $\nu$ . Permeability can be estimated from the grain diameter and porosity of unconsolidated sediments (e.g., using the Kozeny-Carmen equation (Kamaan et al., 2007)) while the kinematic viscosity of water is determined primarily by temperature (Rumble, 2019). Several methods are available for measuring the shear velocity (c.f., Johnson and Cowen, 2017) including a force-balance approach that provides spatially averaged estimates from the depth,  $h_w$ , and slope,  $S$  [-], of a stream:  $u_* = \sqrt{gh_w S}$  where  $g = 9.81 \text{ m s}^{-2}$  is gravitational acceleration. Thus, if the E Profile's two parameters can be expressed in terms of the Permeability Reynolds Number, such relationships might pave the way for a direct translation of laboratory measurements to field applications.

[Figure 8 goes about here]

When log-transformed effective diffusivities are plotted against the log-transformed Permeability Reynolds Number, a significant change in slope and intercept (as represented by non-overlapping 95% confidence intervals in equation (12a)) is evident around  $Re_K = 1$  (**Figure 8a**).

$$\log_{10}(D_{\text{eff},0}^E, \text{ m}^2\text{s}^{-1}) = \begin{cases} (-5.31 \pm 0.09) + (2.53 \pm 0.17) \times \log_{10} Re_K, & R^2 = 0.905, Re_K < 1 \\ (-5.57 \pm 0.06) + (0.99 \pm 0.15) \times \log_{10} Re_K, & R^2 = 0.440, Re_K > 1 \end{cases} \quad (12a)$$

The power-law exponent for the surficial effective diffusivity declines from  $2.53 \pm 0.17$  in the dispersive regime ( $Re_K < 1$ ) to  $0.99 \pm 0.15$  in the turbulent diffusive regime ( $Re_K > 1$ ). Likewise, the scaling behavior of the inverse depth-scale transitions from a

708 constant value ( $a = 10^{1.74 \pm 0.01} \text{ m}^{-1}$ ) to a weak inverse dependence on the Permeability

709 Reynolds Number ( $a \propto \text{Re}_K^{-0.32 \pm 0.08}$ ) (**Figure 8b**):

710 
$$\log_{10}(a_E, \text{m}^{-1}) = \begin{cases} 1.74 \pm 0.01, & \text{Re}_K < 1 \\ (1.69 \pm 0.02) - (0.32 \pm 0.08) \times \log_{10} \text{Re}_K, & R^2 = 0.18, \text{Re}_K > 1 \end{cases} \quad (12b)$$

711 In the dispersive mixing regime ( $\text{Re}_K < 1$ ) the implied mixing depth,  $1/a_E \approx 2 \text{ cm}$ , is  
712 between 4 and 130 times larger than the diameter of the glass spheres that make up the  
713 sediment bed ( $0.150 \leq d_g \leq 5 \text{ mm}$ , depending on the experiment) and about 20 times larger  
714 than an estimate of the depth to which the time-averaged turbulent velocity boundary  
715 layer penetrates into the streambed called the Brinkman Layer thickness,  $\delta_b \approx 20\sqrt{K} = 0.9$   
716 mm (Voerman et al.'s 2017). It is also about ten-fold less than the depth of the sediment  
717 bed ( $d_b = 20 \text{ cm}$ ), implying that the inferred inverse depth scale is not a proxy for bed  
718 depth. These comparisons raise the question: *what is the physical interpretation of the*  
719 *inverse depth-scale?* Based on a model for mass exchange across the SWI by turbulent  
720 pumping, Higashino et al. (2009) reported that, at depths of around 2 cm and for shear  
721 velocities on the lower end of the range employed by Chandler et al. ( $u_* = 0.01$  and  
722  $0.016 \text{ m s}^{-1}$ ), the root mean square vertical velocity of the interstitial pore fluids are  
723  $>10\%$  of their value at the SWI (ibid, Figure 3). Thus, one plausible interpretation is that  
724 the inverse depth-scale represents the surficial region of streambed over which tracer  
725 mass is vigorously mixed by turbulent pumping.

## 726 **6 Conclusions**

727 In this paper we developed and tested a rigorous one-dimensional modeling framework,  
728 based on Duhamel's Theorem, for predicting mass transfer across the SWI and in the

729 benthic biolayer of a turbulent stream. The framework allows for depth-varying  
 730 diffusivity profiles, accounts for the change in porosity across the SWI, and encodes two-  
 731 way coupling across the SWI, in which mass transfer into the water column from the  
 732 sediment bed alters the water column concentration which, in turn, alters the mass flux  
 733 from the water column to the sediment bed, and so on. We applied this theory to an  
 734 extensive set of previously published measurements of turbulent mixing across a flat  
 735 sediment bed in a closed stirred tank (Chandler et al., 2016) with the goal of evaluating  
 736 the performance of four diffusivity depth profiles (C, E, E2M, and C2E Profiles). Key  
 737 findings include: (1) the flux-gradient diffusive model is a reasonable representation of  
 738 turbulent mass transfer across the SWI and in the interstitial fluids of the sediment bed,  
 739 *provided that the vertical structure of the effective diffusivity is correctly specified*; (2)  
 740 Chandler et al.'s experiments are most consistent with an exponentially declining  
 741 diffusivity profile (i.e., the E Profile); (3) values of the E Profile's two parameters  
 742 (effective diffusivity at the SWI,  $D_{\text{eff},0}^E$ , and inverse depth-scale,  $a_E$ ) vary with the  
 743 Permeability Reynolds Number,  $Re_K$ , a dimensionless number that incorporates bed shear  
 744 stress and sediment permeability; (4) the dependence on the Permeability Reynold  
 745 Number changes abruptly at  $Re_K = 1$ , reflecting different modes of mixing below  
 746 (turbulent dispersion,  $D_{\text{eff},0}^E = 10^{-5.31 \pm 0.09} Re_K^{2.53 \pm 0.17}$  and  $a_E = 10^{1.74 \pm 0.01}$ ) and above (turbulent  
 747 diffusion,  $D_{\text{eff},0}^E = 10^{-5.57 \pm 0.06} Re_K^{0.99 \pm 0.15}$  and  $a_E = 10^{1.69 \pm 0.02} Re_K^{-0.32 \pm 0.08}$ ) this threshold value; (5)  
 748 the effective diffusivity's  $1/e$ -folding depth is approximately 2 cm, consistent with the  
 749 hypothesis that solute mixing in the interstitial fluids of the sediment bed is facilitated  
 750 primarily by turbulent pumping; and (6) this  $1/e$ -folding depth is also concordant with

field and laboratory measurements of the benthic biolayer thickness, implying that turbulent mixing, and in particular turbulent pumping, may play an outsized role in the biogeochemical processing of nutrients and other contaminants in streambed and coastal sediments. Studies are presently underway to extend these findings to mixing across the SWI by bedform pumping and bedform turnover with the ultimate goal of informing pollutant fate and transport in coastal sediments and streams at catchment-to-continental scales (Schmadel et al., 2019; Grant et al., 2018; Gomez-Velez et al., 2015; Gomez-Velez and Harvey, 2014).

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**Figure Captions.**

**Figure 1. (a)** An illustration of how water column turbulence can influence mass transport in the benthic biolayer. In this diagram, the benthic biolayer consists of a flat coarse-grained streambed subject to turbulent pumping (traveling pressure wave, dashed blue line), turbulence penetration (red eddies), and a time-averaged turbulent velocity boundary layer that crosses the sediment-water interface (envelope of black arrows). The vertical mass flux  $J(y)$  arising from these turbulence-linked phenomena is assumed to follow the flux-gradient diffusive model (equation (1a)). **(b)** Turbulent mixing across the SWI can be measured in the laboratory using closed systems, such as a stirred tank. Two-way coupling across the SWI is indicated by the two circular arrows.

**Figure 2.** Four functional forms of the effective diffusivity profile  $f(y)$  trialed in this study (equations (8a) through (8d)). Variables represent the depth into the sediment bed ( $y$ ), surficial effective diffusivity (at the SWI,  $D_{\text{eff},0}$ ), a mixing length-scale ( $1/a$ ), and the thickness of enhanced mixing at the surface of the sediment bed ( $\ell_t$ ).

**Figure 3.** The influence of two-way coupling and finite bed depth on the evolution of solute concentration in the **(a)** water column and **(b)** interstitial fluids of the sediment bed, assuming solute is initially present only in the interstitial fluids of the sediment bed and the diffusivity profile is constant with depth (C Profile). See text for details.

**Figure 4.** Chandler et al.'s water (top panels) and sediment (bottom panels) column data compared to infinite bed model predictions for the **(a)** C Profile, **(b)** E Profile, and **(c)** C2E Profile. Black curves are model-predicted solute concentration in the water column (top panel) and at two depths in the sediment bed (15 and 151 mm below the SWI, lower panel). Blue horizontal lines represent the well-mixed (equilibrium) concentration (equation (9a)). Data highlighted in red correspond to Chandler et al.'s Exp ID 20110613.

**Figure 5.** Performance of the C, E and C2E infinite bed models across all 20 of Chandler et al.'s stirred tank experiments. Model performance metrics include: **(a)** coefficient of determination,  $R^2$ ; **(b)** Root Mean Square Error, RMSE; and **(c)** Akaike's Information Criterion, AICc.

**Figure 6.** An evaluation of C2E fitting parameters inferred from Rhodamine concentration in the water column for all twenty of Chandler et al.'s stirred tank experiments. **(a)** Values of C2E's inverse depth scale vary over three orders of magnitude, as illustrated here with a violin plot. The effective diffusivity **(b)** and depth of the constant mixing zone **(c)** inferred from the C2E profile model (vertical axes) are strongly correlated ( $R^2 = 0.93$  and  $0.76$ ) with, respectively, the E Profile's effective diffusivity and inverse decay depth-scale (horizontal axes).

**Figure 7.** A comparison of **(a)** effective diffusivities and **(b)** inverse depth-scales obtained by fitting the E Profile model to water column (vertical axis) or sediment column (horizontal axis) measurements of Rhodamine concentration. The points are colored to indicate the Permeability Reynolds Number.

**Figure 8.** Permeability Reynolds Number scaling of the E Profile's two parameters estimated from Chandler et al.'s water column measurements. The surficial diffusivity **(a)** and inverse decay length-scale **(b)** follow different scaling relationships in the dispersive (blue lines) and turbulent diffusive (green lines) ranges. Dashed curves are 95% prediction intervals.

**Table 1.** Green's functions for various choices of the diffusivity depth profile and a finite or semi-infinite sediment bed.<sup>1</sup>

<b>C Profile, <math>f_c(\bar{y})=1</math></b>	
Semi-infinite Sediment Bed, $\bar{y}=a_c y > 0$ , $\bar{s}=st_t$	
$\tilde{G}(\bar{y}, \bar{s}) = e^{-\bar{y}\sqrt{\bar{s}}}, \bar{y} > 0$	(T-1)
Finite Sediment Bed, $\bar{y}=a_c y > 0$ , $\bar{d}_b = d_b a_c > 0$ , $\bar{y} < \bar{d}_b$ , $\bar{s}=st_t$	
$\tilde{G}(\bar{y}, \bar{s}) = \frac{e^{-\bar{y}\sqrt{\bar{s}}} \left( e^{2\bar{d}_b\sqrt{\bar{s}}} + e^{2\bar{y}\sqrt{\bar{s}}} \right)}{1 + e^{2\bar{d}_b\sqrt{\bar{s}}}}$	(T-2)
<b>E Profile, <math>f_e(\bar{y})=e^{-\bar{y}}</math></b>	
Semi-infinite Sediment Bed, $\bar{y}=a_e y > 0$ , $\bar{s}=st_t$	
$\tilde{G}(\bar{y}, \bar{s}) = \sqrt{e^{\bar{y}}} \frac{K_1(2\sqrt{\bar{s}}e^{\bar{y}})}{K_1(2\sqrt{\bar{s}})}$	(T-3)
<b>E2M Profile, <math>f_{E2M}(\bar{y}) = \begin{cases} e^{-\bar{y}}, &amp; 0 \leq \bar{x} \leq \bar{\ell}_m \\ \bar{D}, &amp; \bar{x} &gt; \bar{\ell}_m \end{cases}</math></b>	
Semi-infinite Sediment Bed, $\bar{y}=a_{E2M} y > 0$ , $\bar{\ell}_m = -\ln \bar{D}$ , $0 < \bar{D} < 1$ , $\bar{D} = \frac{D'_m}{D_{eff,0}^{E2M}}$ , $\bar{s}=st_t$	
$\tilde{G}(\bar{y}, \bar{s}) = \begin{cases} \tilde{G}_1(\bar{y}, \bar{s}), & \bar{y} \leq -\ln \bar{D} \\ \tilde{G}_2(\bar{y}, \bar{s}), & \bar{y} > -\ln \bar{D} \end{cases}$	(T-5a)
$\tilde{G}_1(\bar{y}, \bar{s}) = \sqrt{e^{\bar{y}}} \frac{I_1(2\sqrt{e^{\bar{y}}\bar{s}}) \left[ K_0\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) - K_1\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) \right] + K_1(2\sqrt{e^{\bar{y}}\bar{s}}) \left[ I_0\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) + I_1\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) \right]}{K_1(2\sqrt{\bar{s}}) \left[ I_0\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) + I_1\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) \right] + I_1(2\sqrt{\bar{s}}) \left[ K_0\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) - K_1\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) \right]}$	(T-5b)
$\tilde{G}_2(\bar{y}, \bar{s}) = \frac{e^{-\left(\bar{y} + \ln \bar{D}\right)\sqrt{\frac{\bar{s}}{\bar{D}}}}}{\sqrt{\bar{D}}} \frac{I_1\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) K_0\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) + I_0\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) K_1\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right)}{K_1(2\sqrt{\bar{s}}) \left[ I_0\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) + I_1\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) \right] + I_1(2\sqrt{\bar{s}}) \left[ K_0\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) - K_1\left(2\sqrt{\frac{\bar{s}}{\bar{D}}}\right) \right]}$	(T-5c)
<b>C2E Profile, <math>f_{C2E}(\bar{y}) = \begin{cases} 1, &amp; 0 \leq \bar{y} \leq \bar{\ell}_t \\ e^{-\bar{y}}, &amp; \bar{y} &gt; \bar{\ell}_t \end{cases}</math></b>	
Semi-infinite Sediment Bed, $\bar{y}=a_{C2E} y > 0$ , $\bar{\ell}_t = a_{C2E} \ell_t > 0$ , $\bar{s}=st_t$	
$\tilde{G}(\bar{y}, \bar{s}) = \begin{cases} \tilde{G}_1(\bar{y}, \bar{s}), & \bar{y} \leq \bar{\ell}_t \\ \tilde{G}_2(\bar{y}, \bar{s}), & \bar{y} > \bar{\ell}_t \end{cases}$	(T-6a)

$\tilde{G}_1(\bar{y}, \bar{s}) = \frac{K_1(2\sqrt{\bar{s}}) \cosh[\sqrt{\bar{s}}(\bar{y} - \bar{\ell}_t)] - K_0(2\sqrt{\bar{s}}) \sinh[\sqrt{\bar{s}}(\bar{y} - \bar{\ell}_t)]}{K_1(2\sqrt{\bar{s}}) \cosh(\bar{\ell}_t \sqrt{\bar{s}}) + K_0(2\sqrt{\bar{s}}) \sinh(\bar{\ell}_t \sqrt{\bar{s}})}$	<p>(T-6b)</p>
$\tilde{G}_2(\bar{y}, \bar{s}) = \frac{e^{(\bar{y} - \bar{\ell}_t)/2} K_1(2e^{(\bar{y} - \bar{\ell}_t)/2} \sqrt{\bar{s}})}{K_1(2\sqrt{\bar{s}}) \cosh(\bar{\ell}_t \sqrt{\bar{s}}) + K_0(2\sqrt{\bar{s}}) \sinh(\bar{\ell}_t \sqrt{\bar{s}})}$	<p>(T-6c)</p>

1042 <sup>1</sup>The functions  $K_0$  ,  $K_1$  , and  $K_2$  are Modified Bessel functions of the Second Kind, while  
1043  $I_0$  ,  $I_1$  , and  $I_2$  are Modified Bessel functions of the First Kind.



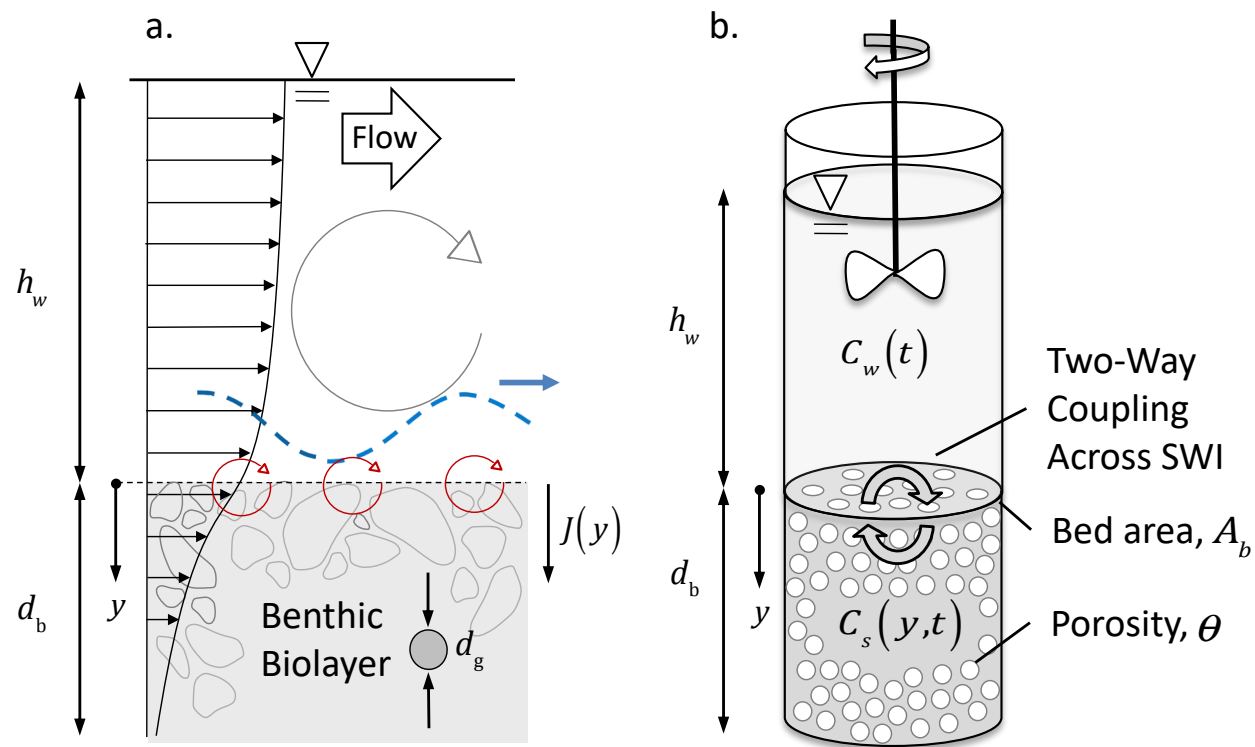


Figure 1. Grant et al. (2020)

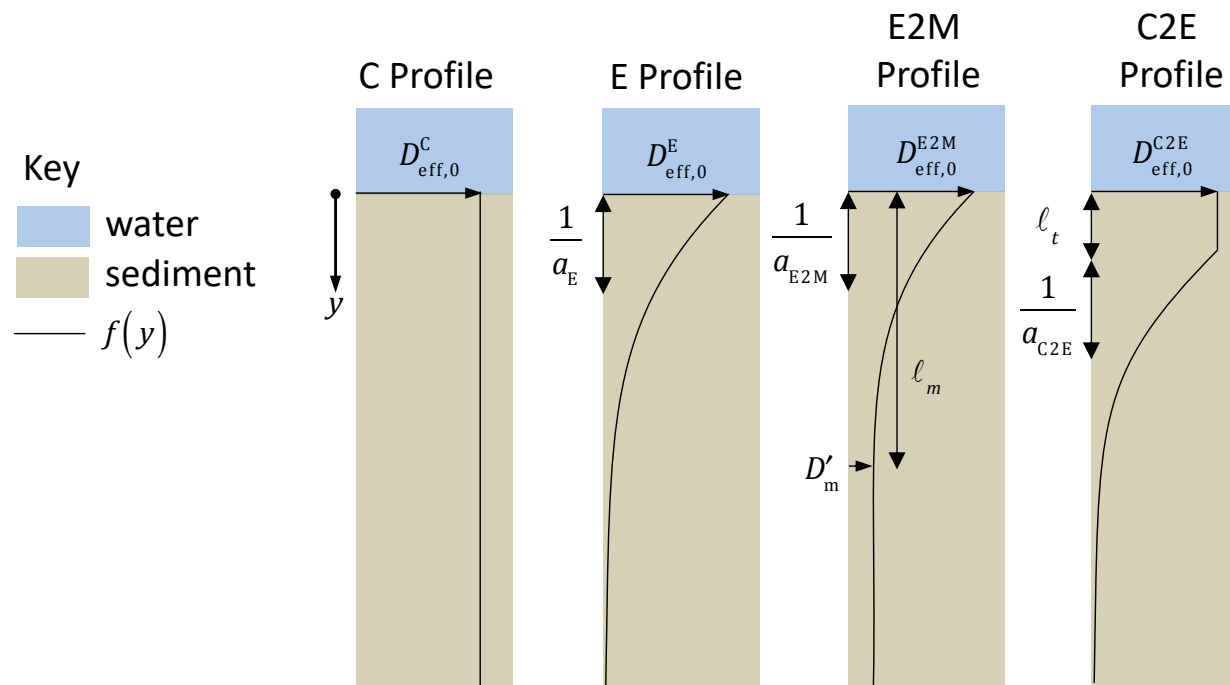


Figure 2. Grant et al. (2020)

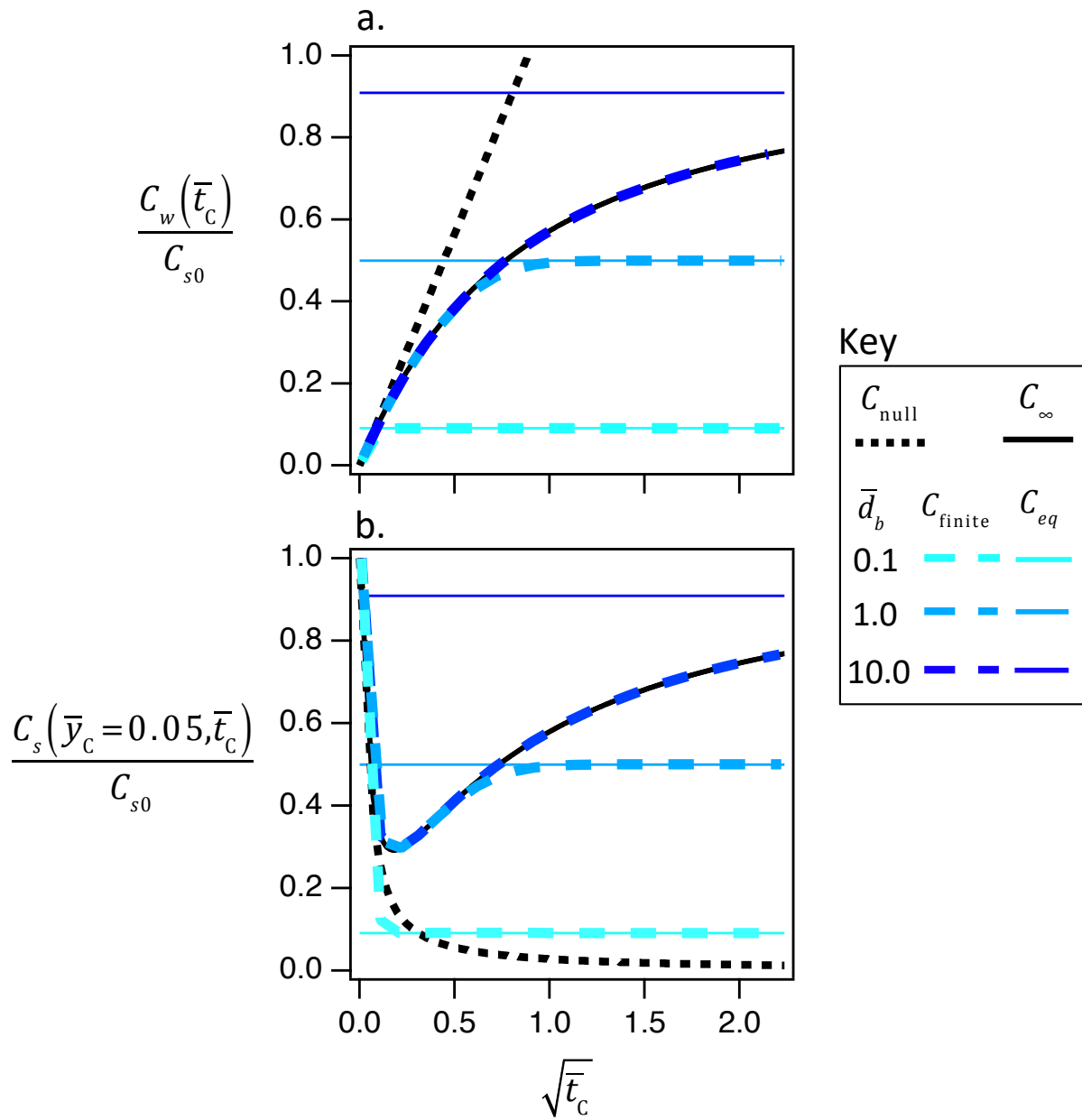


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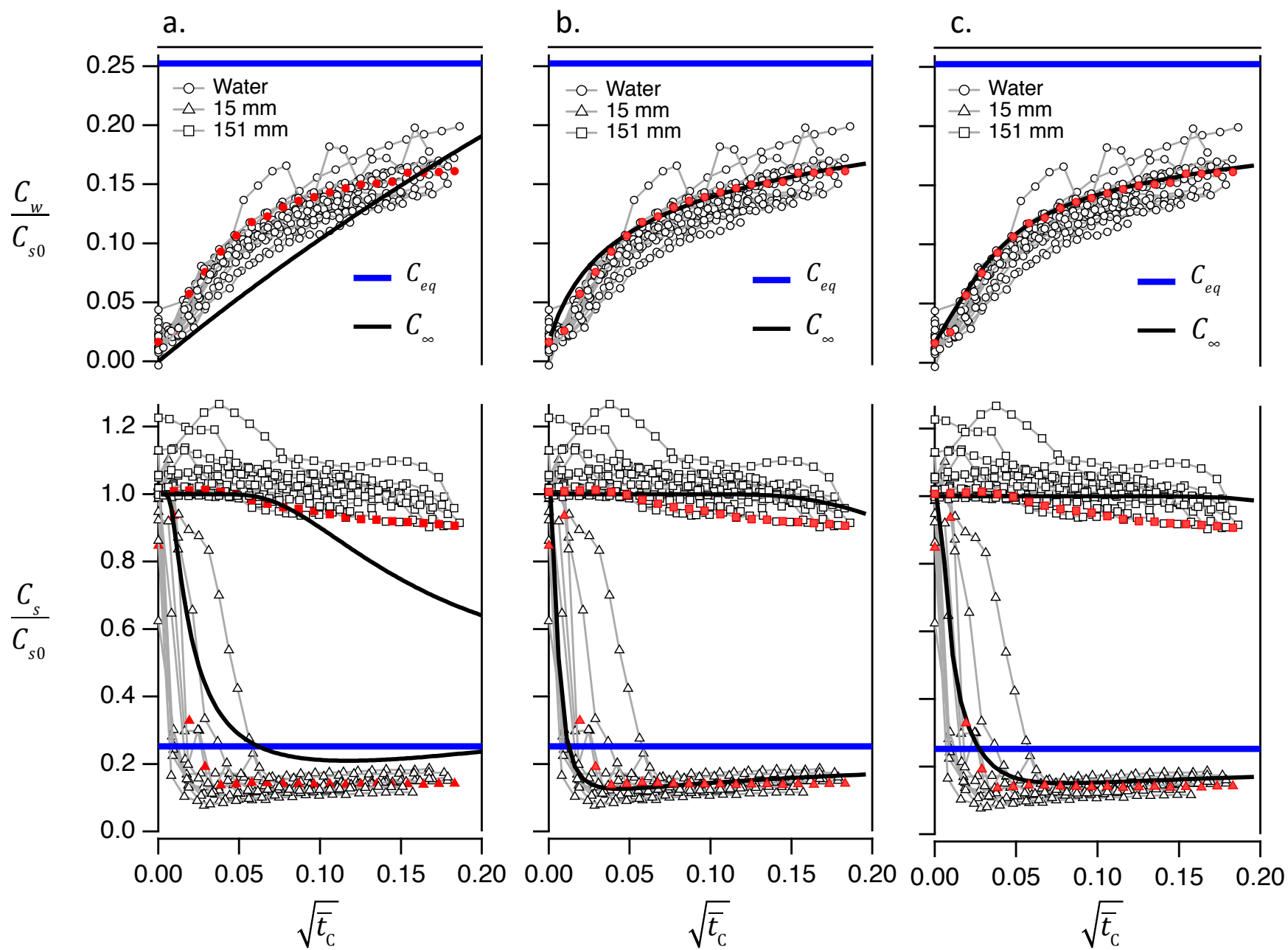


Figure 4. Grant et al. (2020)

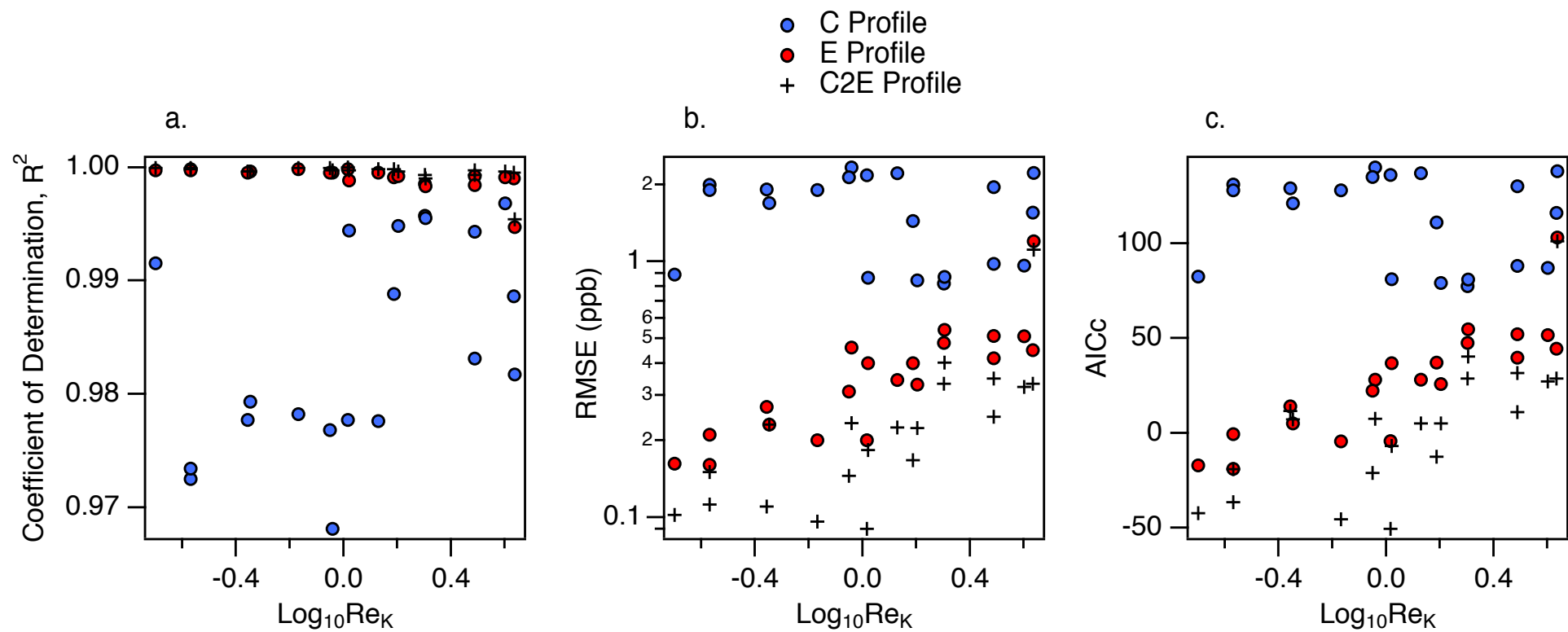


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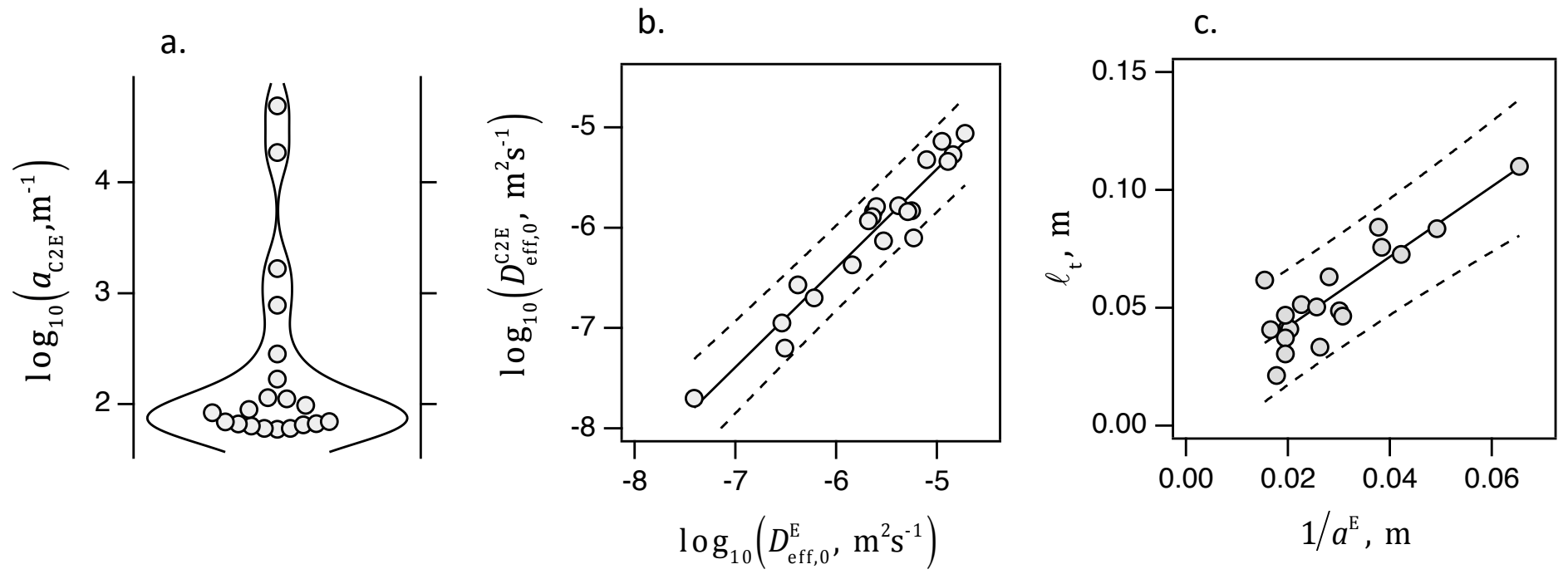


Figure 6. Grant et al. (2020)

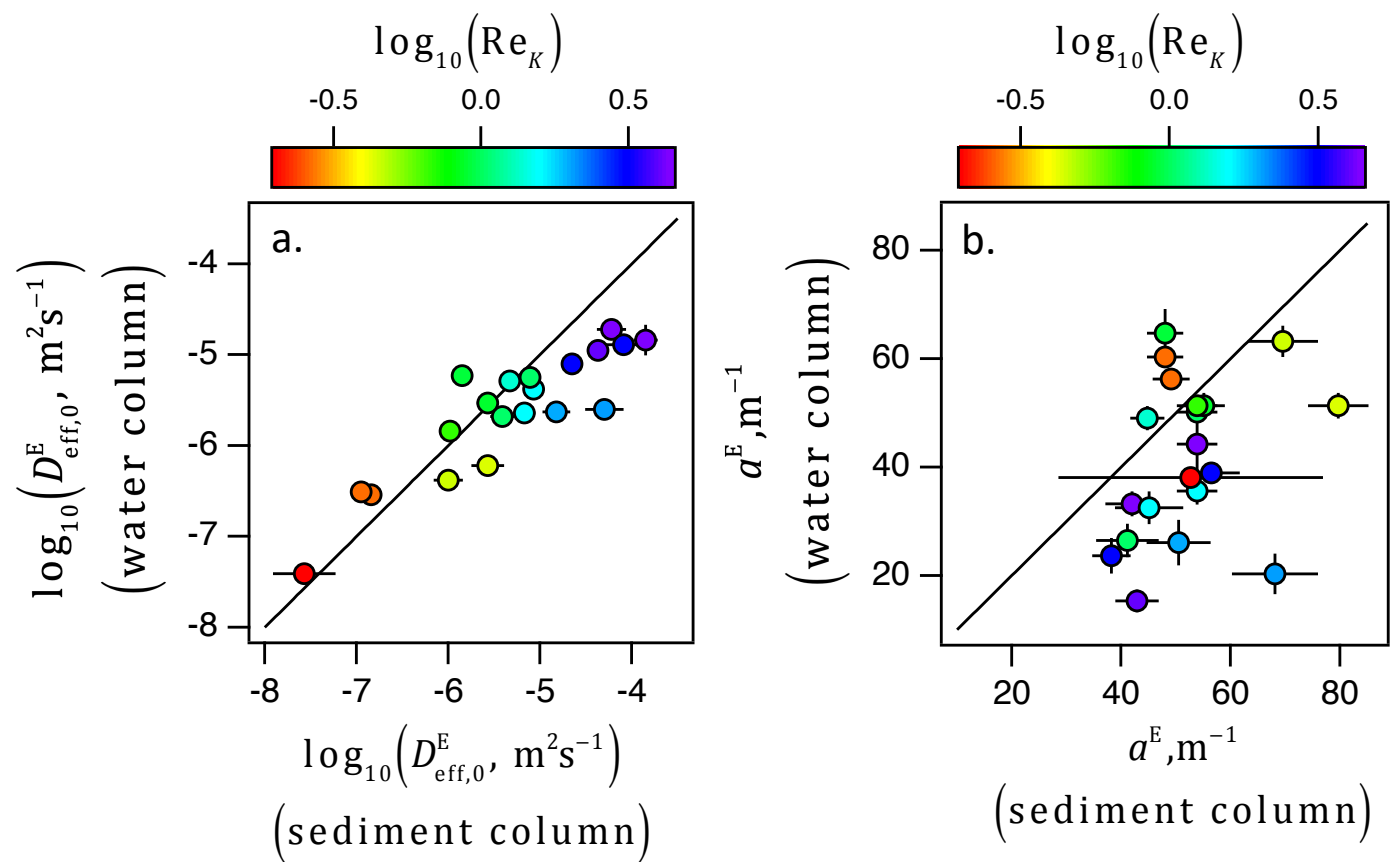


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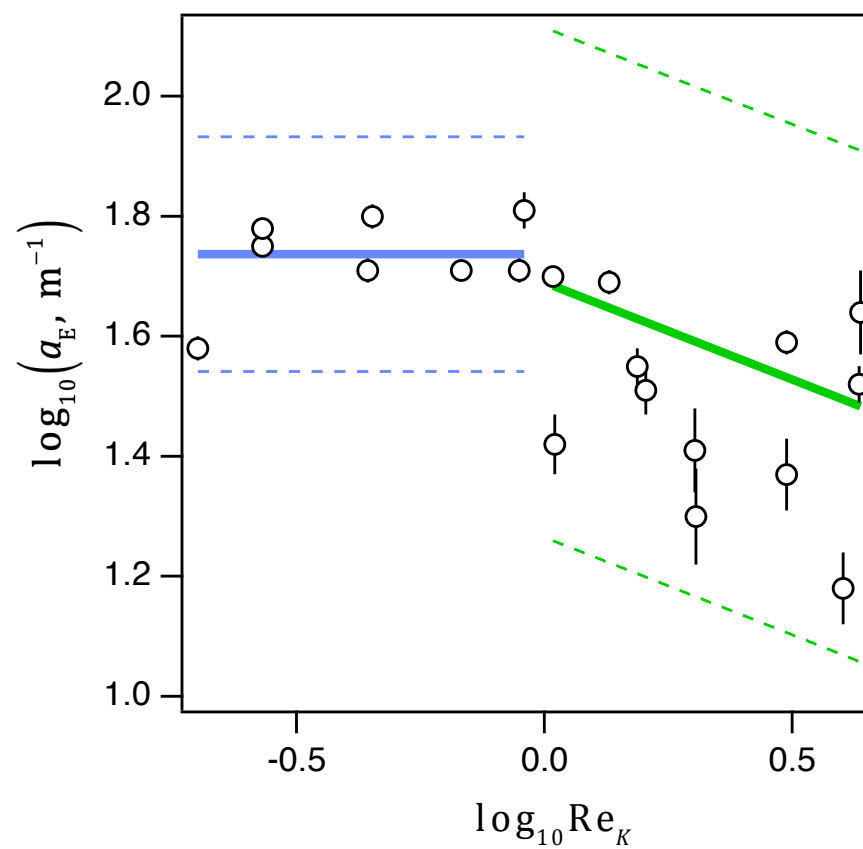
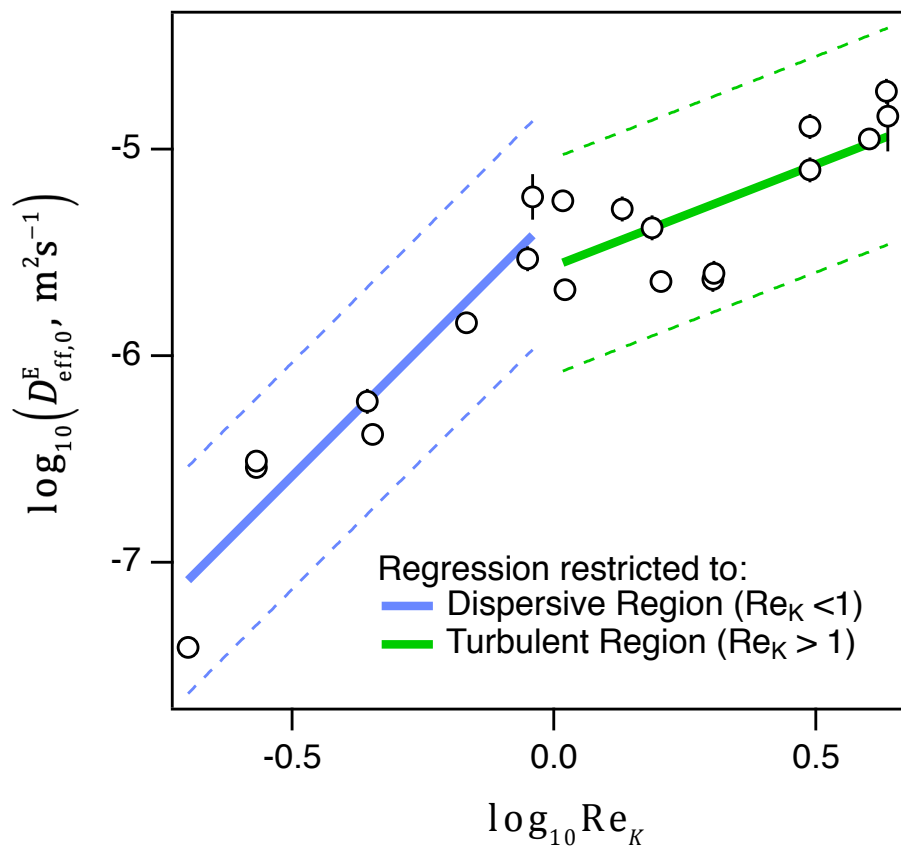


Figure 8. Grant et al. (2020)